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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

TECHNICAL REPORTS
Nos. 1, 3, 5, 6 & 7

MAY 1995

MSL Engineering Limited

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

SPECIFICATION AND PROCEDURE FOR GROUTING OF TEST SPECIMENS

DOC REF C14100R006 Rev 2 JANUARY 1995

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

SPECIFICATION AND PROCEDURE FOR GROUTING OF TEST SPECIMENS

CONTENTS

1. INTRODUCTION

This document presents a detailed procedure for the chord grout-filling of tubular joints to be used as test specimens in a Joint Industry Project (JIP) on the 'Development of Grouted Tubular Joint Technology for Offshore Strengthening and Repair'.

The tubular joints will be used for SCF tests for the ungrouted and grouted conditions. Once tests are complete for the ungrouted condition, grouting can commence in accordance with the specifications and procedure presented within this document.

This document makes reference to the following American Standards:-

- API Specification 10 - Specification for Materials and Testing for Well Cements
- ASTM Specification C150 - Standard Specification for Portland Cement.

2. GROUT FILLING OF SPECIMENS

2.1 General Description

The tubular joint specimens comprise T joints and DT/X Joints. Each of the tubular joints is to be chord grout-filled for SNCF measurements and subsequent ultimate strength tests.

The tubular joints are to be cast with the chord placed in the vertical position. This will ensure complete grout filling of the chord and reduce the number of parameters to consider when interpreting test results. Displacement of water whilst grout filling will be a requirement since grouting offshore in strengthening/repairs will also displace water.

The grout mix and testing specification shall conform to Section 3 herein.

Tubular joints shall be grout-filled using the same procedures, mixing equipment and facilities. This will ensure consistency in grout mix, test cube preparation, grout placement and grout strength once cured. Grout mixer capacity may limit the number of specimens that can be grouted in one operation. In this case measures will be taken to ensure consistency between batches.

2.2 Grout Connections

Grout connection arrangements are shown in Figures 2.1 and 2.2.

For each tubular joint, one inlet shall be provided at the base of the vertical chord and the outlet in either the top cover plate or the top of the chord. The operation of all valves shall be checked, prior to fitting.

All connections shall be well greased. The grout inlet shall be attached to the chord at the grout inlet point. All grout shall be input through this point.

2.3 Filling Chord with Water

The vertical chord members shall be filled with water prior to grout filling. Any leaks identified shall be remedied prior to the grouting operation.

2.4 Grouting the Tubular Joints

This operation shall follow immediately after successful filling of the chord with water.

2.4.1 Mix grout

Grout shall be mixed to a specific gravity of 2.02 ± 0.02 for Oilwell or Portland cement (see Section 3.2 for cement specification). Confirmation of the specific gravity shall be carried out using a pressurised mud-balance. If acceptable, samples will be taken for grout cubes. If the specific gravity is not within the limits specified above, grout shall be mixed until desired density is achieved. Samples for grout cubes will then be taken.

See Section 3 for mixing, sampling and testing of grout.

2.4.2 Grouting operation

- Ensure grout inlet hose is free of any obstructions, 'kinks' or 'crimps' when connected to test specimen.
- Open inlet valves.
- Begin pumping grout through the inlet hose. Pump continuously.
- When good consistency grout flows from the chord outlet point, continue pumping slowly, and take density measurements.
- Following confirmation of satisfactory grout densities, stop pumping, and close inlet valves. Disconnect quick release coupling and reconnect to next specimen. Open inlet valves and begin pumping. When good consistency grout flows from the outlet point, continue pumping slowly and take density measurement. Repeat this cycle for subsequent tubular joints.
- Once all tubular joints are grouted, disconnect grout inlet at quick release union connection, open valve connected to inlet line and pump water down the grout inlet line, to flush.

2.4.3 Short stoppages

If a blockage occurs during grouting of a specimen, adopt the following procedure:-

Stop pumping

Close both inlet valves at inlet point. Disconnect grout line at quick release union connection.

Open grout line inlet valve.

Begin pumping slowly.

If no grout flows, change the inlet grout hose. If grout flows, the problem is not in the hose. Therefore, it is a fault either in the inlet valve, the outlet hose or in the tubular specimen.

Reconnect grout inlet and open inlet valve. Begin pumping. If grout does not flow, then a piece of wire inserted through the outlet point may prove successful in removing any blockage there. If grout still does not flow then the blockage is at the inlet valve or within the tubular specimen and the following course of action may be taken.

- Abort the grouting operation, remedy the fault at the inlet valve or from within the tubular specimen and instigate flushing procedures.

Specimens successfully grouted prior to blockage, shall remain grouted.

2.4.4 Longer stoppages

In the event of a grout flow problem or delay during grouting operations of a specimen, where such delays may exceed one hour, chord flushing procedures must start.

2.4.5 Flushing procedure

Flushing must be carried out if grout flow problems occur which may delay operations for more than one hour.

Specimens successfully grouted prior to blockage, shall remain grouted.

(i) Disconnect grout inlet at quick release union connection, open valve connected to inlet hose and flush inlet hose. Wash out grout mixer.

(ii) Inspect all valves and 'rake out' where necessary.

(iii) Flush specimen through either the inlet or outlet points.

2.5 Post Grouting Procedure

Immediately after satisfactory grouting, close all inlet valves, disconnect at quick release union, open valve connected to inlet hose and flush the grout inlet line.

3. GROUT MIX AND TESTING SPECIFICATION

3.1 Design Requirements

All grout to be used shall achieve a minimum compressive strength of 41.4 N/mm^2 (6000 psi) at 28 days.

3.2 Materials

Cement shall be class 'B' or 'G', moderate sulphate resistant oilwell cement to API Spec 10. Alternatively, moderate sulphate resisting Portland Cement to ASTM C150 Type II may be substituted and used in the same proportions.

Manufacturer's Certificates of Quality with respect to the materials shall be obtained before use.

The cement shall be stored and transported in accordance with the manufacturer's instructions. The cement shall be kept free from moisture at all times and a careful visual inspection of all materials shall be made prior to their use to ensure their suitability for the work. Cement shall be stored out of direct sunlight.

Drinkable water is to be used for mixing, with a temperature not exceeding 20°C.

3.3 Grout Mix Proportions

The grout mix shall be as follows:-

Cement	-	100 parts by weight
Water	-	34 parts by weight (for Oilwell or Portland cement)

NO ADMIXTURES SHALL BE PERMITTED

Figure 3 shows the rate of gain of strength for Oilwell 'B' grouts cured at 8°C (46°F). This is based upon extensive onshore and offshore test data collated from many years of grouting experience.

3.4 **Grout Mixing**

The grout shall be mixed using a suitable mixer (eg. Craelius CEMIX 175 or Colcrete DD4). An initial mix shall be made to line the mixer. This mix shall be discarded. Subsequent batches shall be used to grout the specimens. All batches shall be mixed for a minimum of two minutes.

3.5 **Slurry Density Measurements**

Measurement of slurry densities shall be made using a pressurised slurry density balance in the manner described in API Spec. 10. Particular attention shall be paid to ensure that the external surfaces of the balance are cleaned and dried after filling and prior to balancing.

Grout shall not be pumped until a specific gravity within the limits noted in Section 2.4.1 is achieved. Slurry densities shall be checked immediately prior to pumping and throughout the grouting operations, sampling every batch mixed.

3.6 **Cube Preparation and Curing**

Cubes shall be cast in accordance with API Spec. 10, with the exception that 75mm (3 inch) cubes shall be used.

The cubes shall be placed in polyurethane bags immediately after casting and cured with and at the same temperature as the grouted joints until removed for demolding or testing.

Cubes may be demolded after 24 hours, during which the time out of the bags must not be more than 1 (one) hour. At or after 28 days, cubes shall be weighed, measured and crushed within 30 minutes of removal from the bags.

The cube age shall be measured from the time the cube is struck to the time it is crushed.

Each cube shall be marked with a unique mark and this mark correlated with the batch number, specimen number, time and date made and slurry density, as measured by a pressurised slurry density balance.

3.7 **Sampling and Testing Procedures**

For each batch 4 N° cubes are to be cast from the grout in the grout mixer.

From the 4 N° cubes cast from the grout in the mixer, three (3 N°) cubes shall be tested at 28 days.

An additional 8 N° cubes are to be cast for each grouted test specimen.

From the 8 N° cubes cast with each specimen;

> Three (3 N°) cubes shall be tested at or after 28 days on the commencement of SCF tests on each grouted specimen.
>
> Three (3 N°) cubes shall be tested at the commencement of ultimate strength tests on each grouted specimen.

Each cube shall be crushed in accordance with the procedure given in API Spec. 10, except that the rate of loading will be no faster than 14 N/mm^2 per min (2000 lbf/in^2 per min).

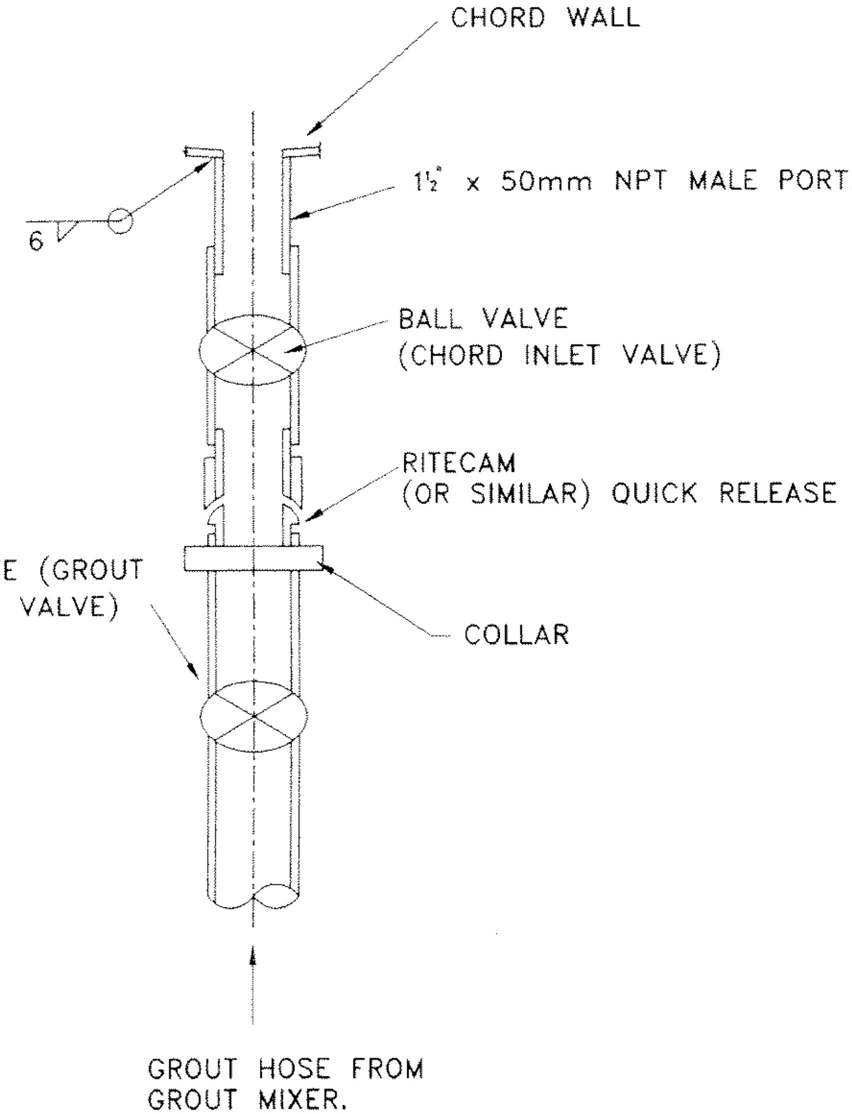
The following information shall be collated for the final report:-

- Test specimen identification reference
- Cube identification reference
- Time and date of casting of the cube and test specimen
- Time and date of testing of the cube and test specimen
- Fluid grout density at time of casting
- Weight and density of the grout cube
- Failure load and cube strength
- Average strength from 3 N° cubes tested at 28 days.
- Average strength from the 3 N° cubes tested at commencement of SCF test on each grouted specimen and 3 N° cubes tested at commencement of ultimate strength test on each grouted specimen.

3.8 Equipment

Calibration certificates are to be supplied for all weighing, balancing, cube making and cube crushing equipment.

FIGURES



NOTES:-
1. DIMENSIONS TO ALLOW CLEARANCES TO OPERATE VALVE HANDLES.
2. ALL VALVES TO BE 1½" BALL VALVES.

FIGURE 2.1. ARRANGEMENT OF CHORD GROUT INLET.

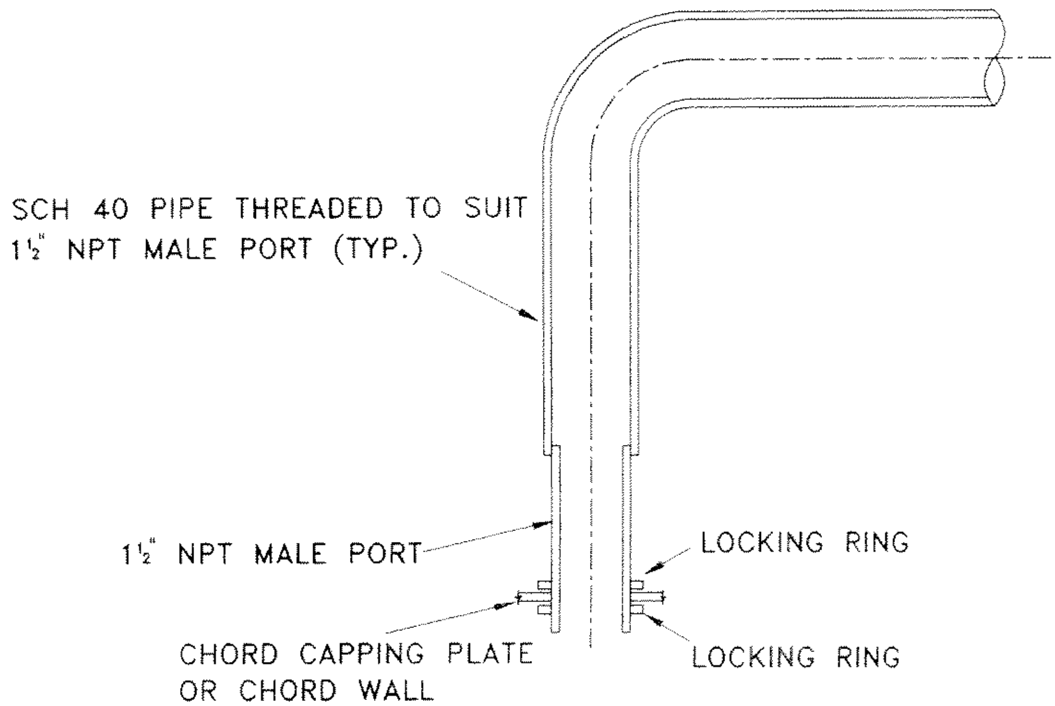


FIGURE 2.2. ARRANGEMENT OF CHORD GROUT OUTLET.

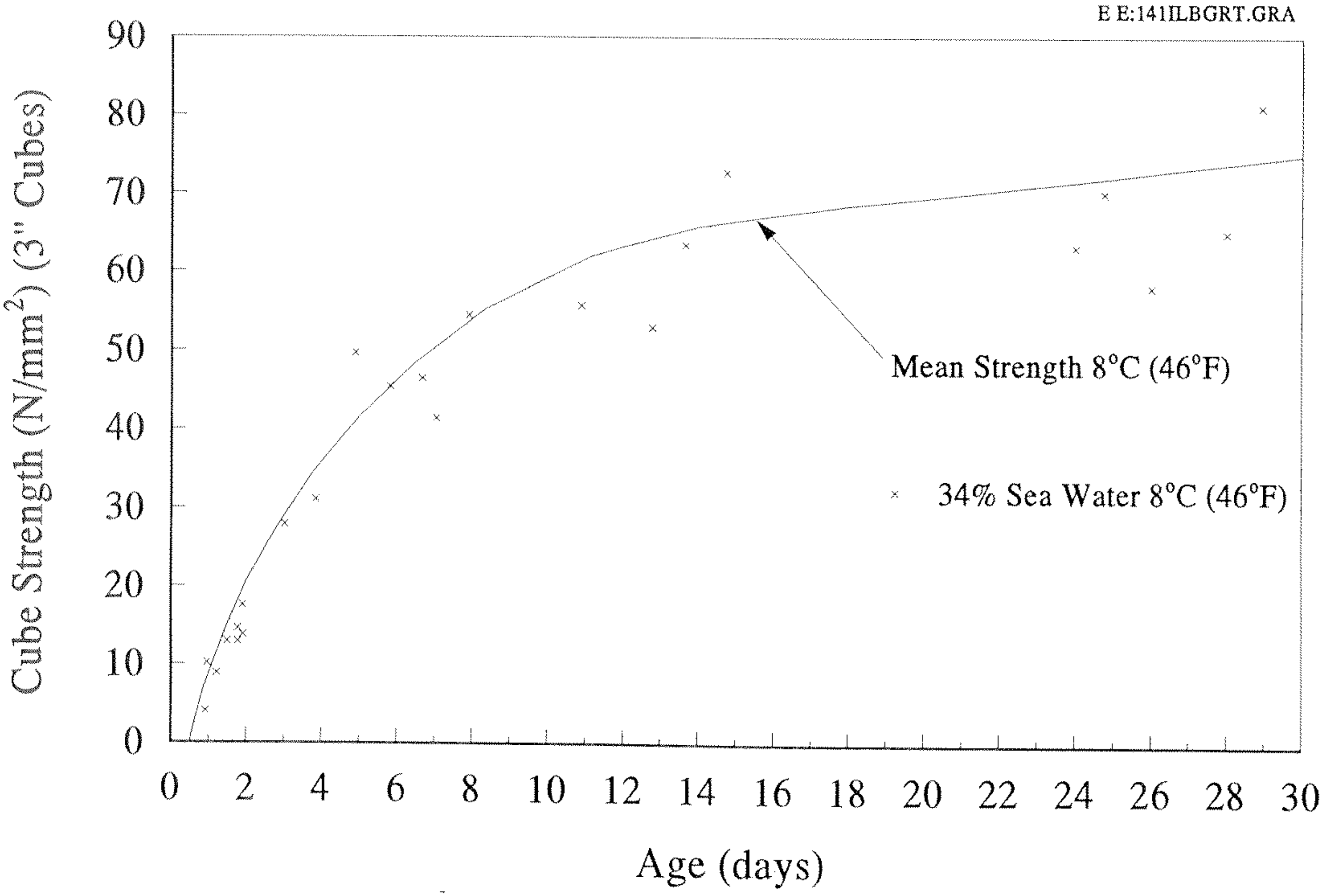


Figure 3 Design Curve for Oilwell B Grout

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

PRE-LOAD INVESTIGATION FOR SPECIMEN T1

DOC REF C14100R008 Rev 2 MAY 1995

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

TECHNICAL REPORT ON THE PRE-LOAD INVESTIGATION FOR SPECIMEN T1

CONTENTS

1. INTRODUCTION

It is proposed that a full pre-load investigation be carried out on specimen T1 (see Appendix A for test specimen matrix). The findings from this pre-load investigation will be used to influence the test procedure for the remaining tests. Issues relating to the pre-load level, sign of load and loading condition are covered within this document.

Section 2 contains a technical appreciation of pre-load effects and reviews previous pre-load investigations conducted by others. Section 3 covers the proposed pre-load regime for specimen T1 and subsequent pre-load procedures for the remaining test specimens.

2. TECHNICAL APPRECIATION

2.1 General

The following subsections present a technical appraisal of the behaviour of grouted tubular joints.

Stress Concentration Factors (SCFs) may be dependent on the previous loading history for grouted tubular joints. This aspect is discussed further in Section 2.3.

2.2 Grouted Tubular Joint Behaviour

The presence of grout significantly stiffens the chord member in the beam bending sense and restricts ovalisation of the chord. The presence of grout has the effect of providing more even distribution of stresses local to the joint intersection and restricts chord wall deformations. However, under tensile loading or on the tension side of in-plane or out-of-plane bending, some level of local separation and yielding may occur, giving rise to the notion of potential SCF dependency on pre-load.

The likely reductions in SCFs for the above mentioned load cases are discussed in the following sections.

2.2.1 Axial Loading

It is expected that the reduction in SCFs, over as-welded SCFs, is greatest at the saddle location since resistance at this location to axial loading is predominantly by chord wall bending for small β ratio joints, and membrane action for high β ratio joints. The presence of grout restricts chord wall deformations and ovality and therefore result in a reduction in SCFs.

SCF reductions for compressive brace axial loading is expected to be greater than tensile loading, as the load transfer mechanism involves bearing onto the grout.

2.2.2 In-plane Bending

The reduction in SCFs for in-plane bending loads is expected to be low, given the greater relative stiffness at crown locations viz a viz saddle locations. The presence of grout causes the neutral axis to shift towards the compressive side of the crown. Therefore, the reduction in SCFs would differ between the tension side and the compressive side of the brace.

2.2.3 Out-of-Plane Bending

The expected reduction in SCFs would be similar to that for axial loading. Therefore, the reduction in SCFs would differ between the tension side and compressive side of the brace. However, with cyclic loading each point will see equal tension and compression loading. The chord provides the greatest resistance at the saddle location for both out-of-plane bending and axial loading.

2.3 **Pre-load Effects**

Pre-load and its magnitude and load sign are expected to affect SCFs for a grouted joint.

Pre-load investigations carried out by Veritec(1) indicated that a threshold SCF value existed for double skin grouted joints. The SCF threshold value is defined as being the highest SCF measured for any pre-load. The SCF threshold value was obtained by steadily increasing tensile pre-load prior to SCF measurements at lower loads, until a drop in the measured SCF was observed. It was observed that substantial yielding occurred at the hot spot location for that preload which gave the SCF threshold value.

The behaviour for fully grouted tubular joints would be expected to be similar to double skin grouted joints.

The Veritec work also measured residual strains as the specimens went through the first SCF measurement cycle. These residual strains increased as the pre-load levels increased. Subsequent shake-down procedures were adopted. A number of cycles, at a load corresponding to a maximum stress within yield at the hot spot location, were conducted until the measured residual strain was less than 1%.

It was found that measured SCFs in specimens which had received reversed pre-loads, ie. tension and compression, were larger than in those which had experienced uni-directional pre-load.

3. TESTING

3.1 Pre-load Investigations on Specimen T1

Examination and assessment of public domain information has indicated that pre-load and the magnitude of pre-load may have a significant effect on the SCF values for a fully grouted tubular joint. It is therefore proposed that the first test specimen (T1) be used to investigate the effects of pre-load. The specimen will be investigated for SCF determination for in-plane, out-of-plane, axial compression and axial tension loading cases, in turn. SCFs will be calculated for loading in the above-noted sequence, since in-plane loading results in the lowest SCFs and axial tension gives rise to the highest SCFs. The proposed test procedure and sequence for specimen T1 are presented in the following table and graphically in Figure 1:

STEP	FORM	LOAD CONDITION
1	Ungrouted	Apply ten cycles of in-plane bending load on the brace at 15% of the predicted ultimate load of the ungrouted joint subjected to in-plane bending. The purpose of this is to 'shake' out residual strains.
2	Ungrouted	Apply in-plane bending loads to the brace in three equal increments, up to a maximum of 15% of the predicted ultimate load of the ungrouted joint subjected to in-plane bending. At each load increment level, measure SNCFs. Reduce the load in three stages, taking SNCF measurements at each load level.
3	Ungrouted	Repeat steps 1 and 2 for out-of-plane bending, axial compression and axial tension, in turn. For each load condition the shake out loading and the limiting load will be taken as 15% of the appropriate predicted ultimate load of the ungrouted joint for the load condition under investigation.
4	Grouted	Apply in-plane bending loads to the brace in three equal increments, up to a maximum of 5% of the predicted ultimate load of the grouted joint subjected to in-plane bending. At each load increment level, measure SNCFs. Reduce the load in three stages, taking SNCF measurements of each load level.

STEP	FORM	LOAD CONDITION
5	Grouted	Repeat step 4 for out-of-plane bending, axial compression and axial tension, in turn.
6	Grouted	Repeat steps 4 and 5, but up to a maximum of 10% of the predicted ultimate load of the grouted joint for the load condition under investigation.
7	Grouted	Repeat steps 4 and 5, but up to a maximum of 15% of the predicted ultimate load of the grouted joint for the load condition under investigation.
8	Grouted	Apply axial pre-load (compression then tension) to the brace member. The magnitude of pre-load shall be 15% of the predicted ultimate strength of the grouted joint under axial tension.
9	Grouted	Remove load from the brace member.
10	Grouted	Repeat step 7.
11	Grouted	Repeat steps 8 and 9, but with a pre-load of 20%.
12	Grouted	Repeat step 7.
13	Grouted	Repeat steps 8, 9 and 7, but with a pre-load of 30%.
14	Grouted	Repeat steps 8, 9 and 7 for 40% and thereafter in 10% increments of the predicted ultimate strength of the joint under axial tension.
15	Grouted	Terminate the test at point of threshold - ie. at point where SNCFs begin to decrease or become constant.

3.2 Pre-load Regime for Remaining Specimens

The pre-load regime for remaining specimens will be determined after completion of test T1 and assessment of the results of this test. It is considered unlikely that the remaining specimens will be investigated to point of threshold.

3.3 <u>Instrumentation</u>

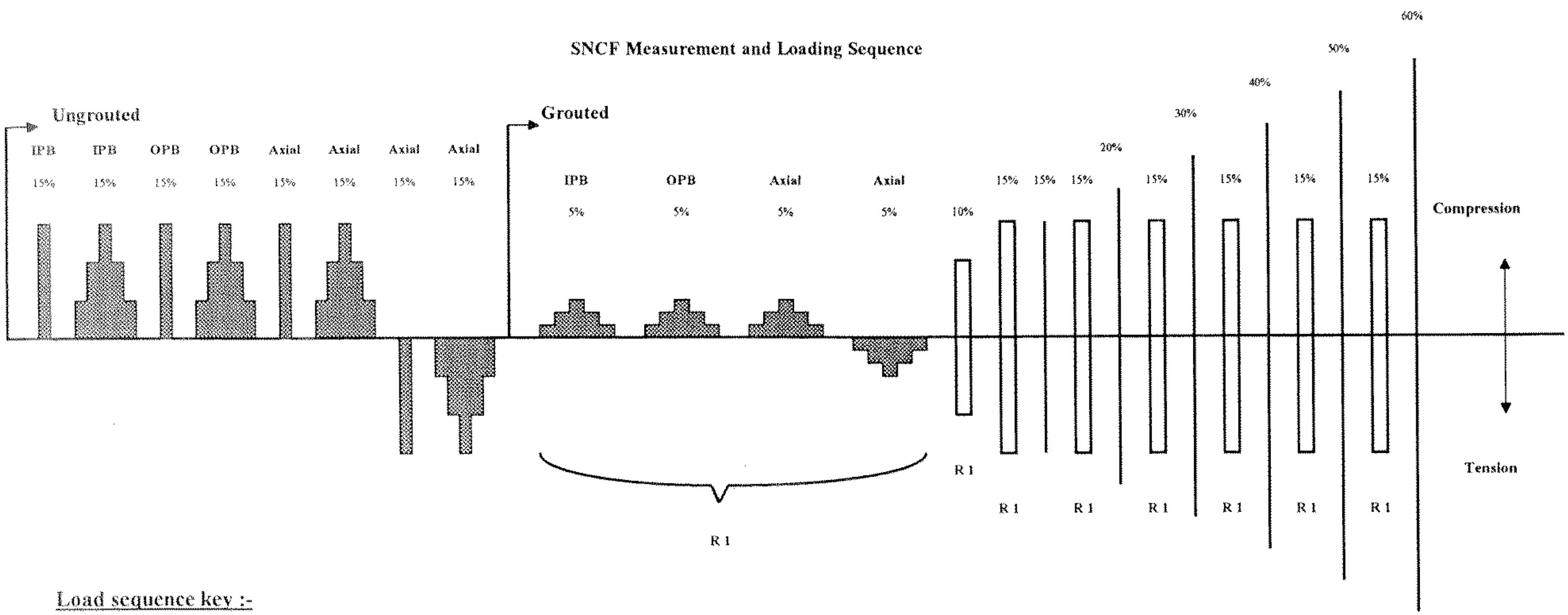
The Veritec work used single filament strain gauges to determine the hot spot location for both the ungrouted and grouted specimens. Once located, the specimens were re-instrumented with single filament and rosette strain gauges.

For this JIP, it is proposed to 'crowd' one quadrant of each specimen with strip gauges. This will ensure that the hot spot is accurately located and enable determination of the hot spot SCF.

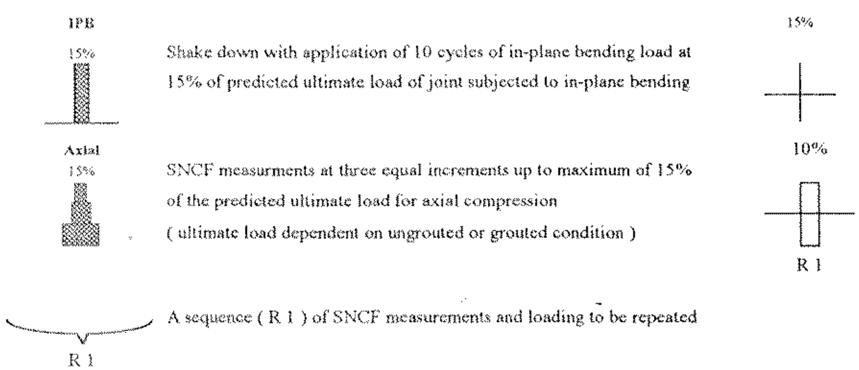
REFERENCES

1. VERITEC, 'Double Skin Grout Reinforced Tubular Joints' Trial Report No. 84-3564, Volumes 1 and 2, November 1984.

SNCF Measurement and Loading Sequence



Load sequence key :-



Shake down with application of 10 cycles of in-plane bending load at 15% of predicted ultimate load of joint subjected to in-plane bending

SNCF measurments at three equal increments up to maximum of 15% of the predicted ultimate load for axial compression (ultimate load dependent on ungrouted or grouted condition)

A sequence (R 1) of SNCF measurements and loading to be repeated

Axial pre-load (compression then tension) at 15% of the predicted ultimate strength of the joint under axial tension acting on brace member

Repeat sequence R 1 at 10% of the predicted ultimate strength

Figure 1 Pre-load Investigations on Specimen T1

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APPENDIX A

TEST SPECIMEN MATRIX

Braces / Chords	$\beta = 0.414$		$\beta = 0.672$		$\beta = 1.0$	
406.4 x 16 $\gamma = 12.7$ Fy = 318	**168.3 x 16** $\tau = 1.0$ Fy = 376	**T1**	**273 x 16** $\tau = 1.0$ Fy = 374	**DT2**	**406.4 x 16** $\tau = 1.0$ Fy = 397	**DT3** **T3**
406.4 x 10 $\gamma = 21.4$ Fy = 345	**168.3 x 10** $\tau = 1.05$ Fy = 395	**DT4**	**273 x 10** $\tau = 1.05$ Fy = 365	**DT5** **T5**	**406.4 x 9.5** $\tau = 1.0$ Fy = 360	**DT6**
406.4 x 7.9 $\gamma = 25.7$ Fy = 351	**168.3 x 8** $\tau = 1.01$ Fy = 278	**T7**	**273 x 7.8** $\tau = 0.99$ Fy = 353	**DT8**	**406.4 x 7.9** $\tau = 1.0$ Fy = 351	**DT9** **T9**

Notes

All units in (N) and/or (mm)

Table 1

Finalised Test Matrix

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

TEST MATRIX

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

TEST MATRIX

CONTENTS

1. INTRODUCTION

This report has been prepared by MSL to inform the PSC of some changes to the original matrix for tests on grouted joints. The primary reason for changes is unavailability of several of the original sizes of steel tubulars. Although the original sizes are standard specification API 5L pipe, or equivalent, considerable sourcing efforts in The Netherlands, UK and Germany have yielded no success. MSL in conjunction with their testing subcontractor, TNO, have therefore prepared a modified matrix for the testing of grouted joints, such that the project objectives will not be compromised.

Revisions to the original test matrix are presented in the following section. The revisions are based on steel tubular sizes for which availability is not a problem. Project objectives with respect to the test programme are also discussed.

2. FINALISED TEST MATRIX

The finalised test matrix is given in Table 2.1. For each test specimen, this table presents details of chord and brace member sizes, Beta, Gamma and Tau factors, along with material specification. Examination of Table 2.1 indicates the following aspects:

- For all test specimens, the chord diameter is 406.4 mm. This is unchanged from the original matrix, ensuring that all tests are conducted at an adequate scale.

- Beta ratios are 0.414, 0.672 and 1.0 compared with 0.4, 0.7 and 1.0 in the original test matrix.

- Gamma ratios range from 12.7 to 25.7 (compared with 15 to 30 in the original test matrix) due to material availability limitations for higher Gamma values. The Gamma range being tested is considered reasonable and will permit certain extrapolation of results beyond the limits.

- The Tau ratios for all test specimens is set at unity, compared to 0.75 in the original test programme. Based on nominal dimensions, test specimens DT4, DT5 and T5 indicate Tau=1.05. The implication of this is expected to be small. The need to change Tau from 0.75 to 1.0 has resulted from availability of required tubular sizes.

  With greater Tau, the brace strength in bending is increased and therefore for ultimate in-plane bending, or out-of-plane bending tests, premature brace failure may be avoided. The effects of Tau variation on SCF calculations will be studied in test specimens DT10 and DT11.

- Geometries for test specimens DT10, DT11 and X1 have not been defined at present. In accordance with PSC comments at the previous meeting, the geometry for these specimens will be defined in consultation with the PSC, once results of the other tests are available.

- Chord material specification is Fe360 steel, or equivalent ($Fy=240N/mm^2$). All braces are Fe510 steel, or equivalent ($Fy=355N/mm^2$) material specification, with the exception of braces for tests T7, T9, DT8 and DT9, which are steel grade Fe360 or equivalent. Material unavailability has been the cause for material specification changes from the original test matrix.

  It is known that SCFs and the location of HSS are independent of steel grade. Therefore change in material specification is of no consequence to achieving the main objective of this project. Material grade (strength) however, does influence ultimate strength, and the proposed matrix should lead to joint failure before brace failure. In this manner, the results from ultimate tests will be more useful since <u>joint failure</u> loads will be determined. Steel tube unavailability in the required sizes for test specimens T7, T9, DT8 and DT9 has dictated use of Fe360 braces.

Table 2.2 has been prepared to indicate predicted failure load for each test specimen. These tables show the joint/brace failure load ratio, and therefore present which ultimate load test (ipb or opb) is possible. The predicted failure loads have been calculate based on actual Mill certificate yield strengths.

REFERENCES

1. JIP on Strengthening, Modification and Repair techniques for Shallow Water and Deepwater offshore Platforms, Design Recommendations Part III, MSL Document Reference C11100R223 Rev. 0, April 1993.

TABLES

Braces / Chords	$\beta = 0.414$		$\beta = 0.672$		$\beta = 1.0$	
406.4 x 16 $\gamma = 12.7$ Fy = 318	**168.3 x 16** $\tau = 1.0$ Fy = 376	**T1**	**273 x 16** $\tau = 1.0$ Fy = 374	**DT2**	**406.4 x 16** $\tau = 1.0$ Fy = 397	**DT3** **T3**
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Notes

All units in (N) and/or (mm)

Table 2.1

Finalised Test Matrix

GROUTED JOINTS JIP

Nominal Pipe Dimensions

Specimen	D	d	T	t	β	τ	γ	Z_p	Yield strength Chord	Yield strength Brace	Moment Capacity kNm Joint[1] IPB	Moment Capacity kNm Joint[1] OPB	Moment Capacity kNm Brace	Brace/Joint Capacity Δ IPB	Brace/Joint Capacity Δ OPB	Ultimate Strength Tests IPB	Ultimate Strength Tests OPB
T1	406.4	168.3	16	16	0.41	1.00	12.7	3.72E+05	318	376	130	130	140	1.08	1.08	Yes	Yes
T3	406.4	406.4	16	16	1.00	1.00	12.7	2.44E+06	318	397	756	1014	969	1.28	0.96	Yes	No
T5	406.4	273	10	10	0.67	1.00	20.3	6.92E+05	345	365	231	233	253	1.09	1.08	Yes	Yes
T7	406.4	168.3	7.9	8	0.41	1.01	25.7	2.06E+05	351	278	71	71	57	0.81	0.81	No	No
T9	406.4	406.4	7.9	7.9	1.00	1.00	25.7	1.25E+06	351	351	412	552	440	1.07	0.80	Yes	No
DT2	406.4	273	16	16	0.67	1.00	12.7	1.06E+06	318	374	341	344	396	1.16	1.15	Yes	Yes
DT3	406.4	406.4	16	16	1.00	1.00	12.7	2.44E+06	318	397	756	1014	969	1.28	0.96	Yes	No
DT4	406.4	168.3	10	10	0.41	1.00	20.3	2.51E+05	345	395	88	88	99	1.13	1.13	Yes	Yes
DT5	406.4	273	10	10	0.67	1.00	20.3	6.92E+05	345	365	231	233	253	1.09	1.08	Yes	Yes
DT6	406.4	406.4	10	9.5	1.00	0.95	20.3	1.50E+06	345	360	513	687	539	1.05	0.78	Yes	No
DT8	406.4	273	7.9	7.8	0.67	0.99	25.7	5.49E+05	351	353	186	187	194	1.04	1.03	Yes	Yes
DT9	406.4	406.4	7.9	7.9	1.00	1.00	25.7	1.25E+06	351	351	412	552	440	1.07	0.80	Yes	No

Worst Tolerance Combination

Specimen	D	d	T	t	β	τ	γ	Z_p	Yield strength Chord	Yield strength Brace	Moment Capacity kNm Joint[1] IPB	Moment Capacity kNm Joint[1] OPB	Moment Capacity kNm Brace	Brace/Joint Capacity Δ IPB	Brace/Joint Capacity Δ OPB	Ultimate Strength Tests IPB	Ultimate Strength Tests OPB
T1	406.4	168.3	18.4	16	0.41	0.87	11.0	3.72E+05	318	376	149	149	140	0.94	0.94	No	No
T3	406.4	406.4	18.4	16	1.00	0.87	11.0	2.44E+06	318	397	870	1166	969	1.11	0.83	Yes	No
T5	406.4	273	11.5	10	0.67	0.87	17.7	6.92E+05	345	365	266	268	253	0.95	0.94	No	No
T7	406.4	168.3	9.085	8	0.41	0.88	22.4	2.06E+05	351	278	81	81	57	0.70	0.70	No	No
T9	406.4	406.4	9.085	9.085	1.00	1.00	22.4	1.43E+06	351	351	474	635	503	1.06	0.79	Yes	No
DT2	406.4	273	18.4	16	0.67	0.87	11.0	1.06E+06	318	374	392	395	396	1.01	1.00	Yes	Yes
DT3	406.4	406.4	18.4	16	1.00	0.87	11.0	2.44E+06	318	397	870	1166	969	1.11	0.83	Yes	No
DT4	406.4	168.3	11.5	10	0.41	0.87	17.7	2.51E+05	345	395	101	101	99	0.98	0.98	No	No
DT5	406.4	273	11.5	10	0.67	0.87	17.7	6.92E+05	345	365	266	268	253	0.95	0.94	No	No
DT6	406.4	406.4	11.5	9.5	1.00	0.83	17.7	1.50E+06	345	360	590	790	539	0.91	0.68	No	No
DT8	406.4	273	9.085	7.8	0.67	0.86	22.4	5.49E+05	351	353	214	215	194	0.91	0.90	No	No
DT9	406.4	406.4	9.085	9.085	1.00	1.00	22.4	1.43E+06	351	351	474	635	503	1.06	0.79	Yes	No

(1) Reference 1, Formulation from Design Recommendations, MSL Document Reference C11100R223 Rev 0, April 1993

Note : Lower table based on 15% increase in Chord thickness and a 0% decrease in Brace thickness
(Allowable tolerance to API 5L Specification for Line Pipe)

Table 2.2

Comparison between Predicted Grouted Joint Strength & Predicted Brace Capacity for Ultimate Load Test

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

STRAIN GAUGING OF TEST SPECIMENS

DOC REF C14100R014 Rev 1 MAY 1995

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

STRAIN GAUGING OF TEST SPECIMENS

CONTENTS

1. INTRODUCTION

The prime objective of the grouted joints testing programme is to measure Strain Concentration Factors (SNCFs) for both ungrouted and grouted T and DT tubular joints of various geometries.

There are three aspects which need consideration to enable the correct measurement of strains. These are as follows:

- adequate number of strain gauges and correct positioning to enable extrapolation of strain to the weld toe.
- adequate number of strain gauges around the circumferential chord/brace intersect to enable interpolation to the hot spot location.
- rosette gauges may be required to enable measurement of principal strains when the principal stress direction is not orthogonal to the chord/brace intersect.

These aspects are addressed in Section 2 which also details current guidance for gauge positions to enable measurement of strains and extrapolation to the weld toe. The proposed strain gauge instrumentation for test specimens in this project is contained in Section 3.

2. BACKGROUND

Determination of SNCFs at the weld toe can be carried out using either linear or non-linear extrapolation of strain measurements. Either method should not be influenced by the stress concentrating effect of the weld. With the exception of K and Y joints (ungrouted), determination of SNCFs in tubular joints can generally be carried out using linear extrapolation. Due to the variation in gauge locations between each method, it is therefore important to either predict which type of extrapolation is required or make provision for both.

Very little test data exist for SNCF measurements on grouted tubular joints. The type of extrapolation to be used is not known and therefore it is necessary to bound the possibility of either linear or non-linear extrapolation.

Table 2.1 presents recommended strain gauge positions for the test matrix based on the following guidance:

- HSE, Reference 1 and ECSC give essentially the same guidance for strain gauge locations to enable linear extrapolation to the weld toe.
- DnV, Reference 2, recommends the first strip gauge location to be 0.25T (where T is the thickness of the tubular) from the weld toe with four subsequent strip gauges at 2 mm centres, for linear extrapolation.
- R S Puthli et al, Reference 3, give guidance for gauge locations to enable either linear or non-linear extrapolation.

All the above state that the first strip gauge should be located a minimum 4 mm from the weld toe in order to avoid the concentrating effect of the weld. The guides attempt to position the gauges in the region of stress linearity, between the region effected by the weld and where the stress becomes equal to the nominal stress.

Puthli et al go one step further by giving guidance for the location of gauges to enable non-linear extrapolation, ie. quadratic extrapolation.

For any of the above methods, extrapolation is made from several strain gauge measurements. This, therefore, will also influence the number of gauges required to enable either linear or non-linear extrapolation.

It is expected that for all specimens within the test matrix for this project, with exception of the X joint, the principal stress direction will be orthogonal (or very close) to the chord/brace intersect. The variation in principal strain is expected to be small and therefore the requirement for rosette gauges is not justified. Since the initial tests are planned for T and DT joints, strain gauging of the X or Y joint will be addressed at a later stage, during the course of the project.

3. PROPOSED GAUGING

Table 3.1 presents the proposed strip gauge positions to enable either linear or non-linear extrapolation. The gauge region, ie. between the first and last gauges, is sufficient to adequately bound the variation and increase in stress towards the chord/brace intersect. The number of gauges, as shown in Figures 3.1 and 3.2, will enable a sufficient number of intermediate readings on both the brace and chord side of the intersection.

It is proposed that the first, middle and last gauges of the strip gauge will be connected. The first gauge position will be 0.4 x (T or t) but not less than 4mm. The HSE recommended first gauge position will coincide or be interpolated within the strip gauge. A single gauge will be placed at the HSE recommended last gauge position. The unused gauges from each strip will act as contingency, should one of the nominated gauges fail to function.

For both joint types it is proposed that the first $\beta = 1.0$ specimen tested will have a greater number of gauges. For the DT joint, one brace will have a full set of gauges around the compressive side of the intersect and the other brace will have a full set of gauges around the tensile side of the interesect (under IPB). The T joint will have gauges around the full intersect. This will enable an assessment of strain distribution around the intersect. It is proposed that the first DT joint tested is a non $\beta = 1$ joint. This particular specimen will be instrumented at all saddle and crown locations. It is expected that symmetry will allow a reduced number of gauge locations for the remaining specimens as proposed below.

Additional strain gauges are provided on the brace(s) to enable measurement of nominal axial strains and brace bending strains. Depending on joint type, gauges will be placed, as follows:

T joints

Saddle and crown locations on both the chord and brace as shown in Figure 3.1. Additionally for $\beta = 1.0$ test specimens, two diagonally opposite quadrants will have a further two sets of gauges, at equal spacing, on both the chord and brace,

DT joints

Saddle and crown locations on both the chord and brace as shown in Figure 3.2. Additionally for $\beta = 1.0$ test specimens, the appropriate quadrant on each brace will have a further two sets of gauges, at equal spacing, on both the chord and brace.

Table 3.1 presents the proposed strain gauge positions for the first and last gauges for each of the specimens. These positions include the HSE recommendations and bound the gauge positions from the other formulations.

REFERENCES

1. "Background to new fatigue design guidance for steel welded joints in offshore structures."
   Report of the Dept. of Energy, Guidance Notes Revision Drafting Panel - 1984.

2. Det norske Veritas "Rules for Classification of Fixed Offshore Installations", Oslo, Norway, July 1993.

3. "Numerical and experimental determination of strain (stress) concentration factors of welded joints between square hollow sections".
   Puthli, R.S., Wardenier, J., Koning, C.H.M de., Wingerde, A. M. van., and Doren, F.J. van.
   Heron, Volume 33, 1988, Number 2.

Specimen Ident.	Geometry				Chord Crown						Chord Saddle						Brace Crown						Brace Saddle					
	D	T	d	t	1st			Last			1st			Last			1st			Last			1st			Last		
	(mm)	(mm)	(mm)	(mm)	HSE	DnV	Puthli	HSE	DnV	Puthli	HSE	DnV	Puthli	HSE	DnV	Puthli	HSE	DnV	Puthli	HSE	DnV	Puthli	HSE	DnV	Puthli	HSE	DnV	Puthli
T1	406.4	16	168.30	16.0	7.3	4.0	6.4	18.3	12.0	16.0	7.3	4.0	6.4	17.7	12.0	16.0	7.3	4.0	6.4	23.9	12.0	16.0	7.3	4.0	6.4	23.9	12.0	16.0
T3	406.4	16	406.40	16.0	11.4	4.0	6.4	22.8	12.0	16.0	11.4	4.0	6.4	17.7	12.0	16.0	11.4	4.0	6.4	37.1	12.0	16.0	11.4	4.0	6.4	37.1	12.0	16.0
T5	406.4	10	273.00	10.0	7.4	4.0	4.0	16.3	12.0	10.0	7.4	4.0	4.0	17.7	12.0	10.0	7.4	4.0	4.0	24.0	12.0	10.0	7.4	4.0	4.0	24.0	12.0	10.0
T7	406.4	7.9	168.30	8.0	5.2	4.0	4.0	12.9	12.0	8.7	5.2	4.0	4.0	17.7	12.0	8.7	5.2	4.0	4.0	16.9	12.0	8.7	5.2	4.0	4.0	16.9	12.0	8.7
T9	406.4	7.9	406.40	7.9	8.0	4.0	4.0	16.0	12.0	8.7	8.0	4.0	4.0	17.7	12.0	8.7	8.0	4.0	4.0	26.0	12.0	8.7	8.0	4.0	4.0	26.0	12.0	8.7
DT2	406.4	16	273.00	16.0	9.3	4.0	6.4	20.6	12.0	16.0	9.3	4.0	6.4	17.7	12.0	16.0	9.3	4.0	6.4	30.4	12.0	16.0	9.3	4.0	6.4	30.4	12.0	16.0
DT3	406.4	16	406.40	16.0	11.4	4.0	6.4	22.8	12.0	16.0	11.4	4.0	6.4	17.7	12.0	16.0	11.4	4.0	6.4	37.1	12.0	16.0	11.4	4.0	6.4	37.1	12.0	16.0
DT4	406.4	10	168.30	10.0	5.8	4.0	4.0	14.5	12.0	10.0	5.8	4.0	4.0	17.7	12.0	10.0	5.8	4.0	4.0	18.9	12.0	10.0	5.8	4.0	4.0	18.9	12.0	10.0
DT5	406.4	10	273.00	10.0	7.4	4.0	4.0	16.3	12.0	10.0	7.4	4.0	4.0	17.7	12.0	10.0	7.4	4.0	4.0	24.0	12.0	10.0	7.4	4.0	4.0	24.0	12.0	10.0
DT6	406.4	10	406.40	9.5	8.8	4.0	4.0	17.8	12.0	10.0	8.8	4.0	4.0	17.7	12.0	10.0	8.8	4.0	4.0	28.6	12.0	10.0	8.8	4.0	4.0	28.6	12.0	10.0
DT8	406.4	7.9	273.00	7.8	6.5	4.0	4.0	14.5	12.0	8.7	6.5	4.0	4.0	17.7	12.0	8.7	6.5	4.0	4.0	21.2	12.0	8.7	6.5	4.0	4.0	21.2	12.0	8.7
DT9	406.4	7.9	406.40	7.9	8.0	4.0	4.0	16.0	12.0	8.7	8.0	4.0	4.0	17.7	12.0	8.7	8.0	4.0	4.0	26.0	12.0	8.7	8.0	4.0	4.0	26.0	12.0	8.7

Strip Gauge Locations (in mm) from weld toe defining regions of strain linearity

Formulation :-

Location	Gauge Position	HSE / ECSC	DnV	Puthli	
				Linear	Quadratic
Chord Crown	1st	$0.2(rt)^{0.5}$	0.25T	0.4T	0.4T
	Last	$0.4(rtRT)^{0.25}$	0.25T + 8mm	1.0T	1.4T
Chord Saddle	1st	$0.2(rt)^{0.5}$	0.25T	0.4T	0.4T
	Last	$(5/360)2\pi R$	0.25T + 8mm	1.0T	1.4T
Brace Crown	1st	$0.2(rt)^{0.5}$	0.25t	0.4t	0.4t
	Last	$0.65(rt)^{0.5}$	0.25t + 8mm	1.0t	1.4t
Brace Saddle	1st	$0.2(rt)^{0.5}$	0.25t	0.4t	0.4t
	Last	$0.65(rt)^{0.5}$	0.25t + 8mm	1.0t	1.4t

Note :-

For all , 1st point , 4mm minimum from weld toe

For Puthli - Linear extrapolation for (T or t) < 10mm Last position = 4mm + 0.6 x (T or t)

Quadratic extrapolation for (T or t) < 10mm Last position = 4mm + 1.0 x (T or t)

For quadratic extrapolation, sufficient number of data points are required between 1st and last

The above dimensions are based on nominal tubular thickness Gauge positions based on actual tubular thickness may vary

Table 2.1 Guidance for location of strain gauges to enable extrapolation of strains to chord/brace wall intersect

Specimen Ident.	Geometry				Chord Crown		Chord Saddle		Brace Crown		Brace Saddle	
	D	T	d	t	1st	Last	1st	Last	1st	Last	1st	Last
T1	406.4	16	168.30	16.0	6.4	18.3	6.4	17.7	6.4	23.9	6.4	23.9
T3	406.4	16	406.40	16.0	6.4	22.8	6.4	17.7	6.4	37.1	6.4	37.1
T5	406.4	10	273.00	10.0	4.0	16.3	4.0	17.7	4.0	24.0	4.0	24.0
T7	406.4	7.9	168.30	8.0	4.0	12.9	4.0	17.7	4.0	16.9	4.0	16.9
T9	406.4	7.9	406.40	7.9	4.0	16.0	4.0	17.7	4.0	26.0	4.0	26.0
DT2	406.4	16	273.00	16.0	6.4	20.6	6.4	17.7	6.4	30.4	6.4	30.4
DT3	406.4	16	406.40	16.0	6.4	22.8	6.4	17.7	6.4	37.1	6.4	37.1
DT4	406.4	10	168.30	10.0	4.0	14.5	4.0	17.7	4.0	18.9	4.0	18.9
DT5	406.4	10	273.00	10.0	4.0	16.3	4.0	17.7	4.0	24.0	4.0	24.0
DT6	406.4	10	406.40	9.5	4.0	17.8	4.0	17.7	4.0	28.6	4.0	28.6
DT8	406.4	7.9	273.00	7.8	4.0	14.5	4.0	17.7	4.0	21.2	4.0	21.2
DT9	406.4	7.9	406.40	7.9	4.0	16.0	4.0	17.7	4.0	26.0	4.0	26.0

Notes :-

(1) All dimensions given in (mm)
(2) First gauge position based on 0.4 times wall thickness with a minimum of 4mm
(2) Last gauge position based on HSE formulation

The above dimensions are based on nominal tubular dimensions. Gauge positions based on actual tubular dimensions may vary.

Table 3.1 Proposed strain gauge locations for 1st and last gauges

FIGURES

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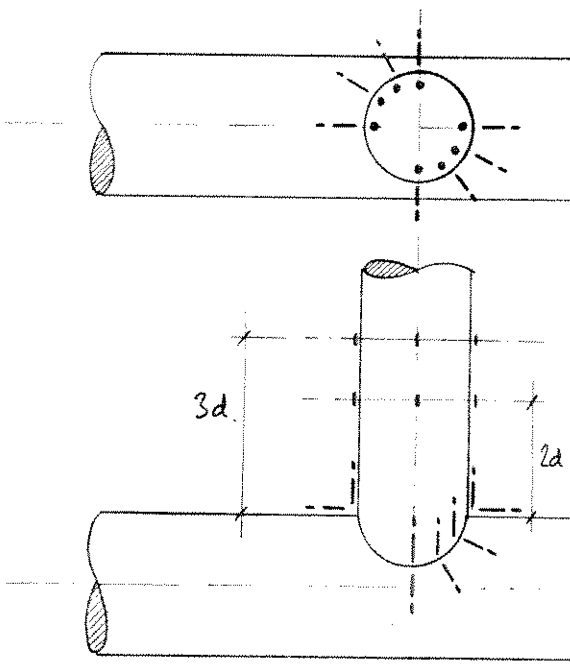
CALCULATION SHEET

MSL Project No. C141	Sheet 1 of 2		Rev. 1
Job Title JIP - GROUTED JOINTS.			
Client —	Report No. C14100R014		
Made by DJM		Date SEPT. '94	
Checked by		Date OCT. '94	

REF



NOTE:- INTERMEDIATE GAUGES @ 30° & 60° POSITIONS ARE FOR $\beta = 1.0$ JOINTS

KEY :-

— STRIP GAUGE, CONSISTING OF 5 STRAIN GAUGES, 3 OF WHICH ARE USED.

- SINGLE STRAIN GAUGE.

N° OF GAUGES REQUIRED PER SPECIMEN

LOCATION.	$\beta = 0.41$ & 0.67		$\beta = 1.0$	
	STRIP GAUGES	SINGLE GAUGE	STRIP GAUGES	SINGLE GAUGE
BRACE	4	12	(+4) 8	(+4) 16
CHORD.	4	4	(+4) 8	(+4) 8
TOTAL.	8	16	(+8) 16	(+8) 24

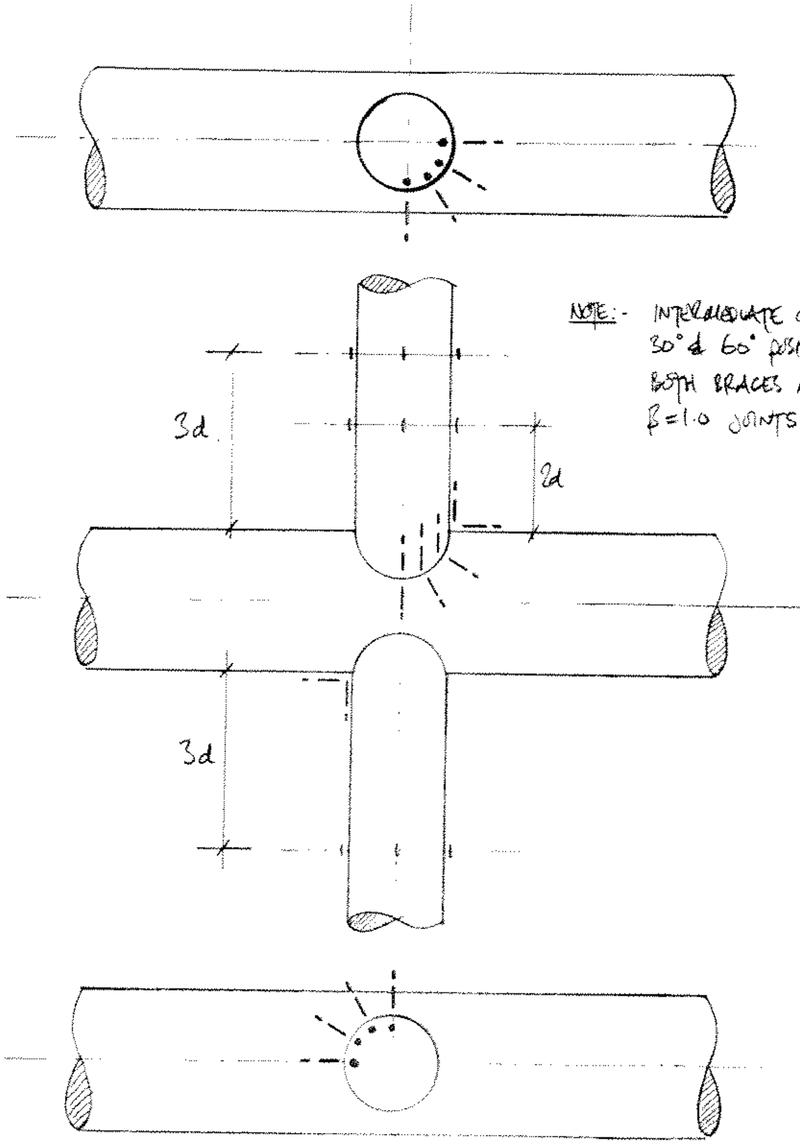
NOTE: THE FIRST $\beta = 1.0$ JOINT TESTED WILL HAVE A FULL COMPLIMENT OF GAUGES, BY PROVIDING ADDITIONAL INTERMEDIATE GAUGES ( ).

FIGURE 3.1

STRAIN GAUGE POSITIONS FOR T JOINTS.

MSL Engineering Limited	MSL Project No. C141	Sheet 2 of 2	Rev. 1
Technology Transfer Centre, Silwood Park, Buckhurst Road, Ascot, Berkshire SL5 7PW	Job Title JIP - GROUTED JOINTS.		
Tel: +44 (0)344-874424 Fax: +44 (0)344-874338	Client —	Report No. C14100R014.	
	Made by DJM	Date SEPT. '94.	
CALCULATION SHEET	Checked by [signature]	Date OCT '94	

REF



NOTE:- INTERMEDIATE GAUGES @ 30° & 60° POSITIONS ON BOTH BRACES ARE FOR β=1.0 JOINTS.

No OF GAUGES REQUIRED PER SPECIMEN.

LOCATION.	β = 0.41 & 0.67		β = 1.0	
	STRIP GAUGES	SINGLE GAUGES	STRIP GAUGES	SINGLE GAUGES
BRACES	4	16	(+6) 8	(+6) 20
CHORD	4	4	(+6) 8	(+6) 8
TOTAL	8	20	(+12) 16	(+12) 28

NOTE: THE FIRST β=1.0 JOINT TESTED WILL HAVE GAUGES AROUND THE COMPRESSIVE SIDE
( ) OF ONE BRACE & GAUGES AROUND THE TENSILE SIDE OF THE OTHER BRACE FOR I.P.B.

FIGURE 3.2

STRAIN GAUGE POSITIONS FOR DT JOINTS.

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

OFFSHORE INSTALLATION STUDY

DOC REF C14100R015 Rev 1 JANUARY 1995

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JOINT INDUSTRY PROJECT

DEVELOPMENT OF GROUTED TUBULAR JOINT TECHNOLOGY FOR OFFSHORE STRENGTHENING AND REPAIR

OFFSHORE INSTALLATION STUDY

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Page

1. INTRODUCTION

This document has been prepared within the Joint Industry Funded Project on Grouted Joints. It presents a collation of the experience gained and procedures used on a number of projects where member grouting has been used as a strengthening measure. Two scenarios are used to illustrate the procedures.

Reference is made to previous offshore projects to evaluate durations of offshore activities which are itemised for use by Participants in preparing timescale and cost estimates for future projects.

There is a certain amount of commonality in the detailed grouting procedures which is dependent of the particular scenario. Other diving related operational matters such as permissible dive times, diving support and detailed equipment needs are outside the scope of this document and so are not addressed

2. SUMMARY

Two repair scenarios are investigated where grouted member technology is used as a strengthening measure. One is horizontal member filling used for joint strengthening, and the other is an inclined member filling used, for example, to reinforce a dented brace member.

Information has been extracted from actual offshore diving logs to prepare timescale estimates for the two scenarios.

Cost estimates have not been prepared because of dependency on many variables such as, time of year, location, diver support vessel (DSV) requirements, number of operations utilising the DSV and complexity of diving operations. However typical cost for a shared DSV in summer could be £50,000 to £100,000 and for a DSV working solely on a job in winter could be £400,000 to £600,000.

Detailed grouting procedures, grout mixing and quality control procedures are provided.

3. SCENARIOS

Two scenarios for grouting of members are studied which represent the most likely applications for the technology offshore. Grouting of legs has not been considered for the following reasons:-

- In many cases there is a pile passing through the leg in which case the chord member is not fully grouted.
- Where there is not a pile inside the legs, many legs are used for storage of water or diesel, and so grout filling would not be an acceptable option.
- Legs may contain ring stiffeners and internal diaphragms at joints which have not been considered in this study in combination with grout filling.
- The volume of grout required is an order of magnitude greater and would require special consideration in regard to the heat of hydration generated.

3.1 Horizontal Member Grouting

Figure 3.1 shows the general arrangement of a horizontal member in a plan level conductor bay which has been grout filled. Grouting would be required in this instance to increase the fatigue life of the joints along the member length. Such a member may have a constant diameter all the way along or there may be expanded joint cans at intervals along the length, see Figure 3.2. This scenario may be taken as being representative of a horizontal member and the procedures may be repeated for similar configurations involving other sizes and lengths of members.

3.2 Inclined Member Grouting

Figure 3.3 shows the general arrangement of a jacket in which inclined member grouting was required to a face frame member due to damage which has occurred during installation. In this instance it may be necessary to fill the whole length of the member or it may only be required to fill the lower portion. Note, however, that the grout filling provided local stability around dents. It was not the intention to increase the axial capacity of the member above its design value. If such a case were required, the procedures proposed would not be satisfactory as they do not result in 100% filling of the member, see Section 4.2.

4. OFFSHORE GROUTING PROCEDURES

4.1 Horizontal Member

4.1.1 General Principles

In order to simplify diver operations it is assumed in these procedures that all access to the inside of the member is gained from the uppermost part of the tube. In this way the diver can sit on the tube whilst working rather than hanging underneath. A problem has now been created since it is not advisable to allow grout to free fall through the flooded member as dilution or segregation of the grout slurry will occur. To overcome the problem, the inlets are extended to within approximately 50 mm of the bottom inside surface of the tube, see Figure 4.1.

Two options have been used in the past to attach the inlets and outlets. The first is to drill and tap holes in the tube with threads which match those of the valves and fittings, see Figure 4.1. The second is to drill or burn and grind dress a hole and clamp the fittings over the hole, as illustrated in Figure 4.2. Drilling and tapping will withstand higher grout pressures but requires greater offshore effort. Clamped inlets and outlets may be more convenient on inclined members where it is difficult to locate and handle the drill framework.

In order to maximise the extent of grout filling, a number of simple rules should be followed:-

1. Displace water rather than air when grouting.

2. Grout from the bottom up; ie. inlet at lowest point of circumference at lower end of member.

3. Grout through one inlet only, otherwise water gets trapped in the middle and a void forms.

4. All grout placed in a single, essentially continuous, operation.

5. Grout should fill member as a wave front by starting at one end of the member and moving away from it.

Outlets and vents should be placed at intervals along the member to allow air to escape but also to allow 'bleed water' to migrate out of the member. Bleed is the phenomenon whereby grout particles in a grout column settle downwards during the setting process leaving a layer of water on the top surface of the column. The extent of bleed in the mixes given in Section 5 is usually less than 0.5%. Outlets and vents often serve a dual purpose as they act as sampling tubes to trap grout for testing. A typical sampling tube is manufactured from 2" NB piping and is 1.2 m (4 ft) long. If such a sampling tube is to contain the likely maximum production of 'bleed water' from a grouted member then the sampling tubes/vents/outlets should have the following distribution.

Member OD (mm)	Length of member per sampling tube/vent (m)
324ϕ	13.15
406ϕ	8.37
457ϕ	6.62
610ϕ	3.71

If the sampling tubes are larger or grout returns are piped to the surface then fewer will be needed. However, in order to comply with the five simple rules above it is still necessary to have a minimum of two outlets per member and two inlets (one to use and one spare in case of blockage or damage). Also a vent/sampling tube shall be placed at each high point, eg expanded joint can, see Figure 3.2 where a pocket of air or water can form.

4.1.2 Seals For Partial Grout Filling

In certain circumstances it may not be desirable to completely grout fill a member as to do so may involve an unduly large volume of grout or the additional weight may not be acceptable. Should this be the case then internal member seals (or plugs) shall be used to confine the grout. The accepted method of achieving this type of seal is with grout bags. Typical details and specifications are provided in Section 4.4.2. Grout bags are deployed through, typically, a 4"ϕ hole in the member and filled with grout to a specified pressure which is then locked into the bag. The grout is allowed to set for approximately 24 hours, before the main grouting operation commences. This technology was originally developed for grouted clamps where an annulus has to be sealed. The material is more robust than neoprene or similar elastomers and can be manufactured in thinner, lighter, more easily handled cross sections, It also has the advantage that pressure does not have to be maintained while the main grouting job is underway. Furthermore, the seal also has the same stiffness and strength characteristics as the body of the grout, which is of particular benefit with clamped repairs.

Particular attention should be taken in the use of internal seals where it is proposed to take grout returns back to the surface for sampling. An internal seal must generate sufficient friction/adhesion to the inside surface of the member so that it can withstand the pressure generated by the head of grout. Where a large pressure would be generated then grouting to surface returns may not be possible, and sample tubes will have to be used.

4.1.3 Grouting Procedure

A typical grouting procedure is given in this section.

The member to be grout-filled shall be fitted with two (2 N°) grout inlet points and three (3 N°) grout outlet points. The general arrangement and locations of inlets and outlets are shown in Figures 3.2,4.1 and 4.3. The locations shall be chosen to avoid circumferential girth welds. Precise details of pipe fittings can be confirmed in liaison with the diving/grouting contractor. However it is not recommended that any smaller fittings than 1½" are used, as large grouting back pressures could be generated due to the resistance to flow of grout in a smaller inlet.

Members shall be fitted with grout inlet and outlet points by drilling and tapping the upper surface of each horizontal member. Similar procedures would be followed if clamped inlets and outlets are used. The method described in this procedure for grouting relies on grout return lines to the surface. However, the use of sample tubes at grout outlet points is permitted within the framework of the procedure described herein.

It is essential that leak testing and grouting operations are immediately consecutive and are not commenced until a suitable weather window is forecast. The required timescale will be reviewed by the Grouting Contractor, who will take the decision to proceed in consultation with the Client.

The diver will operate the valves as instructed by the Grouting Supervisor. Grout should be allowed to issue through the outlet points until good quality grout is observed. As a general rule, there should be a steady and consistent flow of dense grout with no intermingling of sea water and no sign of air bubbles.

The grout inlet points are defined as IP1 and IP2, and the grout outlet points (vents) are defined as OP1, OP2 and OP3, see Figure 3.2. The inlet and outlet valve arrangements are shown schematically in Figures 4.1 and 4.3. Suitable outlet arrangements shall be specified if sampling tubes are adopted.

4.1.3.1 Grout Connections

Grout inlet and outlet points will be fitted with valves as shown in Figures 4.1 and 4.3. In addition, grout inlet points IP1 and IP2 will be fitted with bypass valves. All grout inlet/outlet points will also be fitted with a half 'Weco' coupling to take the grout inlet and vent hoses. Suitable connection and valve details shall be specified at the vent locations if sampling tubes are adopted.

BEFORE FITTING, CHECK THE OPERATION OF ALL VALVES. All connections should be well greased. The grout inlet line shall be attached to the member at grout inlet point IP1 (see Figure 3.2). If grouting operations run smoothly (ie. no blockages occur and weather remains favourable), all grout is input through this point. The grout return lines to the surface shall be attached to outlet points OP1, OP2 and OP3 or; conversely, sampling tubes shall be attached at these locations.

NOTE: If grouting operations have to be abandoned part-way through operations, detailed contingencies are given in Section 4.1.3.13.

4.1.3.2 Leak Testing

NOTE: Before proceeding beyond this point, it is essential that a suitable weather window is expected.

BEFORE ANY PUMPING COMMENCES ENSURE THAT AT LEAST ONE VALVE IS OPEN AT THE MEMBER.

4.1.3.3 Filling Member with Dye

Close all valves at IP2, and open all valves at OP1, OP2 and OP3. Close bypass valve at IP1. Connect inlet grout hose to IP1, and return lines/sampling tubes to OP1, OP2 and OP3.

Pump water containing fluorescent dye, concentration 1:1,000,000, through the grout inlet line. Stop pumping when dye issues from grout inlet points. Close valves at OP1, OP2 and OP3.

Pressurise water to a pressure equivalent to the head of grout for surface return lines or 12m head of grout for sampling tube arrangements. The pressure reading will depend on type of mixer used.

4.1.3.4 Inspection for Leaks

Identify any leak points by evidence of dye. All inlet points and outlet points should be checked. In addition, the intersection welds at the member ends shall be checked. Go to Sections 4.1.3.5 or 4.1.3.6 if leaks noted.

If no leaks are apparent after a period of three minutes, go to Section 4.1.3.7.

4.1.3.5 Remedial Procedure - Leak at Inlet or Outlet Points

Check that valves are closed. Check that valves are correctly attached to the member at the threaded fitting. If leak persists, detach subject valve fitting and remove to surface for inspection and check integrity of threaded grout point in member wall.

4.1.3.6 **Minor Leaks**

If leaks are minor it is probable that they will be self-sealing when grouting commences. Minor leaks are those evidenced by dye locally present in the sea water but not visibly issuing from the leak point. The Grouting Supervisor shall be responsible for confirming any leak as minor and will decide whether remedial action is necessary, prior to grouting. Minor leaks and leaks at member welds may be remedied by sealing the leak point with underwater tape.

4.1.3.7 **No Leaks**

If no leaks are evident, or only minor leaks deemed acceptable by the Grouting Supervisor, a second (confirmatory) inspection shall be made.

4.1.3.8 **Check Valve Operation**

Open the valves in the following sequence, whilst slowly pumping dye water, ensuring in each case an immediate and abundant flow of dye water, before reclosing valve.

Sequence OP1, OP2, IP2 (bypass valve remains closed), OP3.

In the event that no flow occurs at a valve, the valve shall be replaced and the procedure from Section 4.1.3.1 onwards shall be repeated.

4.1.3.9 **Grouting the Member**

This operation should follow immediately after a successful leak test.

It is vital that at least one outlet point is open at all times.

4.1.3.10 **Grouting Procedure**

Before any grout is injected, divers will confirm the valves are set as follows:-

(i) Grout inlet hose line is attached at IP1, inlet valve closed, bypass valve open.

(ii) Grout return hose lines/sampling tubes are attached at OP1, OP2 and OP3, outlet valves open.

(iii) Inlet valve at IP2 open, bypass valve closed.

Thus, valve status required is as scheduled below:-

Inlet Point	Valve Position	Outlet Point	Valve Position
IP1 - inlet valve	Closed	OP1	Open
- bypass valve	Open		
IP2 - inlet valve	Open	OP2	Open
- bypass valve	Closed		
		OP3	Open

Mix Grout

Mix grout to a specific gravity of 2.02 $\pm$ 0.02. Confirm SG=2.02 using a pressurised mud-balance. If acceptable, take a sample for grout cubes. If SG is less than 2.00, continue mixing grout until desired density is achieved and then take sample for grout cubes.

See Section 5 for mixing, sampling and testing of grout.

Valve Operating Sequence

Ensure grout inlet hose from surface to member is free of any obstructions, 'kinks' or 'crimps'.

Pump grout down the inlet hose until the line is full of grout and grout issues from the bypass valve.

Open inlet valve at IP1.

Close bypass valve at IP1.

Pumping is continuous. The Grouting Supervisor will assess when grout samples are to be recovered for density checks, in accordance with the specification noted in Section 5.

When grout issues from IP2, check that at OP1, OP2 and OP3 all valves are open. Close inlet valve at IP2.

Close valve at outlet point OP1.

- When grout issues from the outlet points OP2 and OP3, continue pumping, and take density measurements. Once satisfactory grout has been confirmed from OP2 and OP3, first open valve at OP1, and then, close valves at OP2 and OP3.

- When grout issues from the outlet point OP1, continue pumping and take density measurements.

- Once satisfactory grout has been confirmed from OP1, open valves at OP2 and OP3, and reconfirm satisfactory grout density from all three outlet points.

- Following confirmation of satisfactory grout densities, close inlet valve at IP1, and open bypass valve at IP1.

- DO NOT REMOVE INLET HOSE OR SAMPLING TUBES UNTIL SATISFACTORY GROUT DENSITIES HAVE BEEN CONFIRMED AT ALL THREE OUTLET POINTS OP1, OP2 AND OP3.

- Pump seawater down the grout line, to flush to sea. Disconnect grout inlet hose at IP1 at member and retrieve to surface. DO NOT remove the grout return lines/sampling tubes, as the head of grout serves the required function of surplus 'feed' grout during the initial set phase of the grout in the member. Ensure that all three valves at OP1, OP2 and OP3 are open.

4.1.3.11 **Short Stoppages**

If a blockage occurs during grouting, adopt the following procedure:-

Open bypass valve at IP1.

Close inlet valve at IP1.

If no grout flows, change the inlet grout hose. If grout flows, the problem is not in the hose. Flush the inlet grout line with seawater, detach the Weco coupling from IP1 and reattach at IP2. Close inlet valve at IP2, open bypass valve, fill the hose with grout. When grout issues from the bypass valve, open inlet valve at IP2, close the bypass valve and revert to the procedures in Section 4.1.3.10.

4.3.1.12 **Longer Stoppages**

In the event of a grout flow problem or delay during grouting operations, where such delays may exceed one hour, member flushing procedures must start.

4.1.3.13 **Flushing Procedure**

Flushing must be carried out if grout flow problems occur which may delay operations for more than one hour.

(i) Open bypass valve at IP1, shut inlet valve at IP1, and flush grout hose to sea. Wash out grout mixer.

(ii) Pumping seawater, open and close the valves in the following sequence. Do not close a valve until grout has ceased to issue from the valve.

(iii) Open inlet valves at IP1 and IP2. Close bypass valve at IP2. Flush for 15 minutes.

(iv) Close by-pass valve at IP1.

(v) Close inlet valve at IP2 and open all remaining outlet valves and flush for a minimum of 15 minutes. Open inlet valve at IP2, and continue flushing for a further 10 minutes.

(vi) Diver to inspect all valves and 'rake out' where necessary.

4.1.3.14 **Post Grouting Procedure**

Flush Grout Inlet Line

Immediately after satisfactory grouting, flush the grout inlet line (via bypass) and retrieve to surface.

Return Lines/Sampling Tubes and Valve Assemblies

The return lines/sampling tubes and valves will remain in situ until the grout has achieved initial set. This will be agreed with the Client, but will not be less than 24 hours after completion of grouting. Retrieve return lines/sampling tubes and valves thereafter and place thread protector caps over inlets/outlets.

Underwater tape will be wrapped around exposed inlets/outlets if appropriate to prevent corrosion of the threads.

4.2 Inclined Member

The general principles for grouting horizontal members outlined in Section 4.1.1 are equally applicable to inclined members. Since the member is inclined there is no clear advantage to divers working from the uppermost surface of the member and so inlets will be placed directly on the underside of the member.

Intermediate outlets are not necessary as a vertical path is already provided for air and 'bleed water' to escape. Also fewer inlets are provided as the extreme low point of the void is comparatively small unlike a horizontal member. Sealing is provided by the lower and upper brace member welds and no further sealing is considered.

A particular difficulty arising with inclined members is getting grout to fill all the way to the top. It is not possible either due to space restrictions or geometrical limitations to place the uppermost grout outlet exactly at the top of the member. Consequently, a void is formed by the air or water which is trapped above the outlet, see Figure 4.4. This is the reason why, in Section 3.2, it was stated that these procedures are not appropriate where the axial capacity needs to be increased above the design value. Figure 4.5 shows how the use of a weep hole can reduce the void size, (but not eliminate it completely). The following procedures are written assuming a weep hole is used, but are equally applicable if one is not.

The general arrangements of inlets and outlets are shown in Figures 4.2 and 4.6. The precise locations shall avoid circumferential girth welds, anodes and any other surface attachments.

Members shall be fitted with grout inlet and outlet points. The method described in this procedure for grouting relies on grout return lines to the surface. Where appropriate, at the upper end of each member, a small weep hole is drilled in the brace wall which permits a greater degree of member filling than would otherwise be possible. The procedures assume that only one inlet is provided and all work is carried out through a single point. It may in some circumstances be prudent to provide a second inlet to be used as a contingency.

It is essential that leak testing and grouting operations are immediately consecutive and are not commenced until a suitable weather window is forecast. The required timescale will be reviewed by the Grouting Contractor, who will take the decision to proceed in consultation with the client.

Grout Mix and Grouting Specifications are contained in Section 5.

The diver will operate the valves as instructed by the Grouting Supervisor. Grout should be allowed to issue through the outlet points (and weep holes where appropriate) until good quality grout is observed. As a general rule, there should be a steady and consistent flow of dense grout with no intermingling of sea water and no sign of air bubbles.

The grout inlet and outlet points are shown in Figures 4.2 and 4.6. Weep hole details are shown in Figure 4.5.

4.2.1 Member Preparation

4.2.1.2 General

Holes in the grout inlet and outlet points shall be placed at the locations indicated on Figure 4.7. Details of grout inlets and outlets are shown in Figures 4.2 and 4.6. Details of the inlet point indicated in Figure 4.2 shows a 6 o'clock position with respect to the brace circumference. As an alternative, a 12 o'clock position can be adopted, provided an insert pipe is specified to ensure that grout injection takes place close to the 6 o'clock location. No inlet point shall be placed within 300mm of a girth weld or tubular joint weld. In addition, no inlet point shall be placed at any longitudinal seam weld location. If the inlet point is closer than 150mm from a longitudinal seam weld, the inlet point position shall be rotated to give a minimum circumferential separation of 150mm from the longitudinal seam weld.

Outlet points shall be placed at the highest end on the member circumference, at the locations defined, except that no outlet point shall be placed within 300mm of a girth weld or tubular joint weld. In addition, no outlet point shall be placed at any longitudinal seam weld location. If the outlet point is closer than 75mm from a longitudinal seam weld, the outlet point position shall be rotated to give a minimum circumferential separation of 75mm from the longitudinal weld.

Where appropriate, near the outlet point position, a small (ie. 12mm max diameter) weep hole is to be drilled in the brace wall, at the location shown in Figure 4.5. As an alternative, the hole can be made using a cutting rod, provided that a smooth profile free of burrs, notches and molten spatter is achieved. No longitudinal or rotational displacement of the weep hole is permitted.

The purpose of the weep hole is to permit trapped air and seawater to escape.

At each inlet and outlet location, where an inlet/outlet clamp is to be placed, all longitudinal seam welds and other surface irregularities shall be ground down to give a flush surface to the brace wall on which to place the grouting clamp. This will minimise any leakage problems associated with the clamp-member interface.

4.2.1.3 Weep hole sealing

After making the weep hole, it shall be sealed by hammering in a rubber bung or similar device. If necessary, during leak testing and grout curing, the bung shall be held in place by a nylon strap passed around the brace. The bung shall be removed during grout injection procedures to allow grout-filling to the required level; the bung shall thereafter be replaced to permit grout pressure to build and ensure grout returns to the surface.

4.2.2 Grouting Procedure

Grout inlet and outlet points will be fitted with valves as shown in Figures 4.2 and 4.6. In addition, grout inlet and outlet points will be fitted with bypass valves. All grout inlet/outlet points will also be fitted with a half 'Weco' coupling to take the grout supply and return hoses.

BEFORE FITTING, CHECK THE OPERATION OF ALL VALVES.

All connections should be well greased. The grout inlet line shall be attached to the member at the grout inlet point. If grouting operations run smoothly (ie. no blockages occur and weather remains favourable), all grout is input through this point. At the grout outlet, return lines to the surface shall be attached to the outlet valve.

NOTE: If grouting operations have to be abandoned part-way through operations, detailed contingencies are given in Section 4.2.2.11.

4.2.2.1 Leak Testing

NOTE: Before proceeding beyond this point, it is essential that a suitable weather window is expected.

BEFORE ANY PUMPING COMMENCES ENSURE THAT THE OUTLET VALVE IS OPEN AT THE MEMBER.

4.2.2.2 Filling Member with Dye

Open valve at the outlet point. Close bypass valve at outlet point. Close bypass valve at inlet point. Connect inlet grout hose to inlet point, and return line to outlet point.

Pump water containing fluorescent dye, concentration 1:1,000,000, through the grout inlet line. Stop pumping when dye issues from grout outlet point. Close outlet valve.

Pressurise water to a pressure equivalent to the head of grout held in the surface return line. The pressure reading will depend on type of mixer used.

4.2.2.3 Inspection for Leaks

Identify any leak points by evidence of dye. All inlet points and outlet points should be checked. In addition, the damage locations, weep hole seals (where appropriate) tubular joint welds and girth welds shall be checked. Go to Sections 4.2.2.4 or 4.2.2.5 if leaks are identified.

If no leaks are apparent after a period of three minutes, go to Section 4.2.2.6.

4.2.2.4 Remedial Procedure - Leak at Inlet or Outlet Points

Check that valves are closed. Check that valves are correctly attached to the grouting clamp at the fitting. If leak persists, detach subject valve fitting and remove to surface for inspection. Check integrity of the grouting clamp gasket seal to member wall, tighten studbolts if necessary.

4.2.2.5 Minor Leaks

If leaks are minor, it is probable that they will be self-sealing when grouting commences. Minor leaks are those evidenced by dye locally present in the sea water but not visibly issuing from the leak point. Major leaks are those evidenced by a jet of dye flowing from the defective spot. The Grouting Supervisor shall be responsible for confirming any leak as minor and will decide whether remedial action is necessary, prior to grouting. Minor leaks may be remedied by sealing the leak point with underwater tape.

4.2.2.6 No Leaks

If no leaks are evident, or only minor leaks deemed acceptable by the Grouting Supervisor, a second (confirmatory) inspection shall be made.

4.2.2.7 Check Valve Operation

Open the outlet valve whilst slowly pumping dye water, ensuring an immediate and abundant flow of dye water, before reclosing valve.

In the event that no flow occurs at a valve, the valve shall be replaced and the procedure from Section 4.2.2.1 shall be repeated.

4.2.2.8 Grouting the Member

This operation should follow immediately after a successful leak test.

It is vital that the outlet point is open at all times.

4.2.2.9 Grout Injection

Before any grout is injected, divers will confirm the valves are set as follows:-

(i) Grout inlet hose line is attached at inlet point, inlet valve closed, bypass valve open.

(ii) Grout return hose line is attached at outlet point, outlet valve open, bypass valve closed.

(iii) Remove seal from weep hole (if appropriate).

Thus, valve status required is scheduled below:-

Inlet Point	Valve Position	Outlet Point	Valve Position
Inlet valve	Closed	Outlet Valve	Open
Bypass Valve	Open		

Mix Grout

Mix grout to a specific gravity of 2.02 ± 0.02. Confirm this specific gravity using a pressurised mud-balance. If acceptable, take a sample for grout cubes. If the specific gravity is less than 2.00, continue mixing grout until desired density is achieved and then take sample for grout cubes.

See Section 5 for mixing, sampling and testing of grout.

Valve Operating Sequence

- Ensure grout inlet hose from surface to member is free of any obstructions, 'kinks' or 'crimps'.
- Pump grout down the inlet hose until the line is full of grout and grout issues from the bypass valve.
- Open inlet valve at inlet point.
- Close bypass valve at inlet point.
- Pumping is continuous. The Grouting Supervisor will assess when grout samples are taken for density checks, in accordance with the specification noted in Section 5.
- When good consistency grout issues from the weep hole, check that the outlet valve is open. SEAL THE WEEP HOLE AT THIS STAGE.
- When grout issues from the return line at the surface, continue pumping slowly, and take density measurements.
- Following confirmation of satisfactory grout densities, close inlet valve at inlet point, and open bypass valve at inlet point. Ensure that outlet valve is open. Take sample for grout cubes from remaining grout in the grout mixer.

- DO NOT REMOVE INLET OR OUTLET HOSE UNTIL SATISFACTORY GROUT DENSITY HAS BEEN CONFIRMED AT OUTLET POINT.

- Pump seawater down the grout inlet lines, to flush to sea. Disconnect grout inlet hoses at Weco union connection locations, and retrieve to surface. Attach 'feed' hose at the outlet point to the leg using lashing material. The grout in this 'feed' hose will provide a head of grout during the initial grout set phase and permit surplus 'bleed' water to migrate out of the member.

4.2.2.10 Short Stoppages

If a blockage occurs during grouting, adopt the following procedure:-

Open bypass valve at inlet point.

Close inlet valve at inlet point.

If no grout flows, change the inlet grout hose. If grout flows, the problem is not in the hose. Therefore, it is either a fault in the inlet valve or a blockage in the grouting clamp. In either event, the grouting operation must be aborted so that the inlet valve and grouting clamp can be inspected and the fault remedied.

4.2.2.11 Longer Stoppages

In the event of a grout flow problem or delay during grouting operations, where such delays may exceed one hour, member flushing procedures must start.

4.2.2.12 Flushing Procedure

Flushing must be carried out if grout flow problems occur which may delay operations for more than one hour.

(i) Open bypass valve at inlet point, close inlet valve at inlet point, and flush inlet hose to sea.

(ii) Pumping seawater, open and close the valves in the following sequence. Do not close a valve until grout has ceased to issue from the valve.

(iii) Open inlet valve at inlet point. Open outlet valve at outlet point. Close bypass valve at inlet point. Flush for 15 minutes using the outlet point to pump seawater. Repeat flushing for

15 minutes using the inlet point to pump seawater. Do not stop flushing until grout has ceased to issue.

(iv) Inspect all valves and 'rake out' where necessary.

(v) If, as a result of stoppage, the inlet hole becomes permanently blocked by grout, a new inlet hole should be cut in the member as close to the existing hole as possible but avoiding any solid grout. The location of solid grout can be determined by using ultrasonic techniques or, alternatively, by tapping the member with a hammer (a grouted section will sound more solid than an ungrouted flooded section). The new hole should be placed using the same procedures as the original hole.

4.2.2.13 Post Grouting Procedure

Flush Grout Lines

Immediately after satisfactory grouting, close inlet and outlet valves and flush the grout inlet lines (via bypass) and retrieve to surface.

'Feed Hose'

The 'feed' hose at the outlet point shall be attached to the leg using lashing material. Underwater tape will be wrapped around exposed inlets/outlets and weep holes to prevent ingress of seawater. The 'feed' hose may be removed after 24 hours if required.

4.3 Fatigue Details

At completion of the work the inlets and outlets which remain form stress raisers which must be checked against fatigue. Assuming that the holes have been cut in parent plate (not weld metal) in the brace, then a type 'C' S-N curve applies and a hole gives rise to a stress concentration factor of 3.0. At single sided closure welds without thickness transitions a SCF of 1.4 applies with a type 'F2' S-N curve. At double sided butt welds without thickness transitions a SCF of 1.3 applies with a type 'E' S-N curve.

Figure 4.8 illustrates the relative endurance of each of the three details described. It is clear that the grout inlet/outlets represent the worst case. The single sided and doubled sided weld cases will have been checked in detailed design, it is, therefore, necessary to additionally check the brace member for fatigue at the inlet/outlet points.

4.4 Construction Details

4.4.1 Grout Inlets/Outlets

A typical arrangement of a jig used to drill and tap holes in members on an offshore structure is depicted in Figure 4.9. This particular jig is intended to be held in place and react the drilling force by being strapped to the member. However, the jig could equally be held in place by a simple hydraulic grabber arm arrangement. The drill and tap are held against the member by turning the handle at the end of the threaded bar using either a diver or a ROV. It is possible to carry out the drilling and tapping with a single tool in a single operation.

4.4.2 Grout Bags

It is conceivable, in certain circumstances, that it may not be desirable to grout the whole length of the member. Such an occasion would be where additional weight of fill grout would distress the member. In such an event it is necessary to locally confine the grout and this is achieved by the use of grout bags.

Figure 4.10 illustrates the general arrangement of such an application which consists of the following stages:-

1. Cut 125mm x 100mm holes in member.
2. Dress holes to remove sharp edges, burrs etc.
3. Insert grout bags and tie off inlets.
4. Inflate grout bags with 'standard' grout mix.
5. Maintain recommended pressure whilst grout sets.
6. Wait 24 hours after grouting of bags before grouting the void created in the member

The grout bag producers have now developed a standard specification for the material as follows:-

The material is manufactured from yellow woven polypropylene with the following characteristics in a standard tensile test

Warp 310 Kgf per 5 cm width
Weft 175 Kgf per 5 cm width
Weight = 300 g/m^2

The material is inert and is non degradable.

The material shall be protected at all times from prolonged exposure to direct sunlight.

All cutting of the material shall be by the 'hot-knife' method to prevent fraying of the seams and edges. Sewing thread shall be Polyamid Zwylon.

5. GROUT MIXES

This section describes the minimum technical requirements necessary for the grout constituent materials, mix design and quality control for grout filling of tubular members.

5.1 Design Requirements

All grouts to be used shall achieve a minimum compressive strength of 41.4 N/mm^2 at 28 days.

5.2 Grout Mix Proportions

All procedures in Section 4 are based on the grout mix given in Section 5.2.1. However the mix in Section 5.2.2 is equally acceptable, but produces a slightly less dense mix.

5.2.1 Oilwell Cement

The grout mix shall be as follows:-

Class 'B' or 'G' oilwell cement to API Specification 10 or equivalent	100 parts by weight
Water	34 parts by weight

Alternatively, sulphate resisting Portland Cement to specification ASTM C150 Type II may be submitted in the same proportions.

No admixtures shall be permitted.

The mix will produce a slurry with a theoretical specific gravity of 2.05 and a yield of 0.65 m^3 per tonne of cement.

5.2.2 Dutch Encillite

The grout mix shall be as follows:-

Dutch Encillite (Dutch Portland Class B)	100 parts by weight
Water	39 parts by weight

No admixtures shall be permitted.

The mix will produce a slurry with theoretical specific gravity of 1.98 and a yield of 0.7 m^3 per tonne of cement.

5.3 Materials

Manufacturer's Certificates of Quality with respect to the materials shall be obtained before use.

The cement shall be stored and transported in accordance with the manufacturer's instructions. The cement shall be kept free from moisture at all times and a careful inspection of all materials shall be made prior to their use to ensure their suitability for the work.

Potable water shall be used in preference to seawater if readily available on the platform. Seawater shall be clean, if used, and should be taken from the hydrant ringmain no earlier than one hour before mixing. The temperature of the water shall be in the range 4°C to 8°C.

All measures shall be taken to keep the cement and water cool prior to mixing. The measures proposed shall be subject to Client approval.

5.4 Grout Mixing

The grout shall be mixed using a continuous recirculating jet type mixer. The mixer type specified shall be subject to Client approval. An initial mix shall be made to line the mixer. This mix shall be discarded. Subsequent batches shall be used to grout the members. All batches shall be mixed for a minimum of two minutes.

5.5 Quality Control Requirements

5.5.1 Equipment

Calibration certificates are to be supplied for all relevant equipment.

5.5.2 Slurry Density Measurements

Measurement of slurry densities shall be made using a pressurised slurry density balance in the manner described in API Specification 10. Particular attention shall be paid to ensure that the external surfaces of the balance are cleaned and dried after filling and prior to balancing.

Grout mixes in accordance with Section 5.2.1 shall not be pumped until a specific gravity of 2.00 or greater is achieved. Grout mixes in accordance with section 5.2.2 shall achieve a specific gravity of 1.96. Slurry densities shall be checked immediately prior to pumping and throughout the grouting operations, sampling every batch mixed.

5.5.3 Cube Manufacture, Curing and Testing

Cubes shall be manufactured in accordance with Specification BS 1881 Part 108: 1970, or equivalent, with the exception that 75mm cubes shall be used.

The cubes shall be placed in a thermostatically controlled curing tank immediately after manufacture and cured at a temperature of $8°C \pm 1°C$ until removed for demolding or testing. Seawater shall be used in the cube curing/storage tanks.

Cubes may be demolded after 24 hours and returned to the curing tank. Cubes shall not spend more than 1 (one) hour out of the curing tank.

Cubes shall be prepared for testing, weighed, measured and crushed within 30 minutes of removal from the curing tank.

The cube age shall be measured from the time the cube enters the tank to the time it is crushed.

Each cube shall be crushed in accordance with the procedure given in API Specification 10, with the exception that the rate of loading will not exceed 14 N/mm^2 per minute.

Each cube shall be marked with a unique mark and this mark correlated with the jacket member number, time, date made and slurry density as measured by a pressurised slurry density balance.

5.5.4 Sampling and Testing Procedure for Each Member

Four (4 N°) 75mm cubes shall be cast from the grout in the grout mixer at the start of the grouting operation after the minimum specific gravity is achieved.

Grouting operations shall continue until the density of the vented grout from the member achieves the required specific gravity as measured using the method described in Section 5.5.2. Once the specified grout density has been achieved from all vents, four (4 N°) 75mm cubes shall be cast from the remainder of the grout in the mixer using the method described in Section 5.5.3.

In addition to the casting of eight cubes noted above, twenty two (22 N°) 75mm cubes shall be cast during the grouting operation. Sampling shall be carried out at approximately equal intervals throughout the grouting operation.

Cubes shall be tested in accordance with the method described in Section 5.5.3. Three good cubes shall be tested at each of the following intervals:-

- 3 days
- 7 days

- 14 days
- 21 days

The remaining cubes, of which at least six shall be good, shall be tested at 28 days. Any cubes to be tested onshore can be packed in insulated containers for shipment, provided always that the curing regime is maintained.

For each grouted member, the following information shall be collated for the final report:-

- Member identification reference.
- Cube number and position in grouting sequence (eg. beginning, middle, end).
- Time and date of casting the cube.
- Time and date of testing the cube.
- Fluid grout density at time of casting.
- Weight and density of the grout cube.
- Failure load and cube strength.
- Average strength for cubes tested at the same time.

5.5.5 Transportation of Test Cubes

A period of three days shall elapse before the cubes can be made ready for transportation ashore.

Cubes should be demolded and each cube must be individually wrapped in hessian sacking or cloth and placed in a polythene bag. Fresh water should then be added to keep the cube moist during transit. The polythene bag must be then sealed to entrain the water. A label identifying the cube should then be securely attached to the bag.

When all the cubes have been wrapped, they should be packed in a sturdy container. If possible, they should be further cushioned by placing rags, or a similar packing material between individual cubes and around the sides of the container. Any voids remaining after the cubes have been packed should be filled with rags, and finally, a lid should be firmly attached to the container. Attach 'Fragile' packing labels to container.

The cubes should be transported ashore by the fastest possible means. Notification to the consignee must be made in advance in order that the cubes can be uplifted from their location and taken to the testing laboratory for storage. On arrival at the laboratory the cubes shall be immediately placed in a curing tank at a temperature of 8°C ± 1°C until removed for testing.

6. EQUIPMENT LISTS

Equipment requirements to install a grouted member repair are given below. Equipment related to diving support is not included.

Drilling and Tapping

- Drilling and Tapping Jig to cut 1½" - 2" diameter holes in tubulars underwater. (see Figure 4.10)

Grouting

- Continuous batching recirculating jet mixer able to deliver approximately 4 m^3/hour of mixed grout.
- grout holding tank
- grout pump
- cement silo(s)
- 2" grout hose
- 2" ball values and unions (quick connect/disconnect type).

Grout Laboratory

- Curing tank capable of sustaining 8°C + 1°C. Suitable for minimum 30 cubes.
- Pressurised mud balance to test grout slurry density in accordance with API Spec 10.
- 75 mm cube moulds in accordance with BS 1881 part 108:1970.
- Weighing scales
- Callipers for measuring cube dimensions
- Cube crushing machine, minimum 50 tonnes capacity.
- Crates and packing in which to transport cubes ashore.

7. ESTIMATED OFFSHORE DURATIONS

Offshore logs have been made available to MSL Engineering for a project involving grout filling of 2 horizontal members through drilled and tapped inlets and outlets. The following durations are based on this project and do not allow for weather downtime or mechanical breakdown.

<u>Day 1</u>

Setting up equipment.
Start drilling and tapping holes. (10 No total)
Make trial mix.

<u>Day 2</u>

Continue drill and tap.
Make up grout hoses.

<u>Day 3</u>

Complete drill and tap.
Deploy hoses to Member 1.
Leak test Member 1.

<u>Day 4</u>

Grout Member 1.
Recover hoses and fittings from Member 1.
Deploy hoses and fittings to Member 2.
Grout Member 2.

<u>Day 5</u>

Recover hoses and fittings from Member 2.
Demobilise dive team.

<u>Day 6</u>

Cube demoulding.
Demobilise grouting spread.

<u>Day 7</u>

Crush 3 day cubes.

Day 8

Demobilise grout team.

Specific tasks have the following durations:

Drill and tap 10 NO 1½" inlets/outlets	- 30 hours diving
Leak test and grout Member 1 (18.5 m^3 grout) and recover lines	- 14½ hours diving
Leak test and grout Member 2 (11.2 m^3 grout) and recover lines	- 8 hours diving

Work was executed by an air diving team with three divers in the water for a maximum of 180 minutes per dive.

For the two scenarios being addressed, time estimates have been developed using data derived from the reference offshore project.

Horizontal Member

Volume to be filled = $16m^3$

Day 1

Mobilise and set up equipment, make trial mix

Day2

Drill and tap 5 No inlets/outlets, 15 hours diving

Day 3

Deploy lines, grout member and recover lines, 15 hours diving

Day 4

Demobilise diving spread. Clean and service grouting spread. Demould cubes and prepare to transport ashore

Demobilise grouting spread

Day 6

Test 3 day cubes onshore

FIGURES

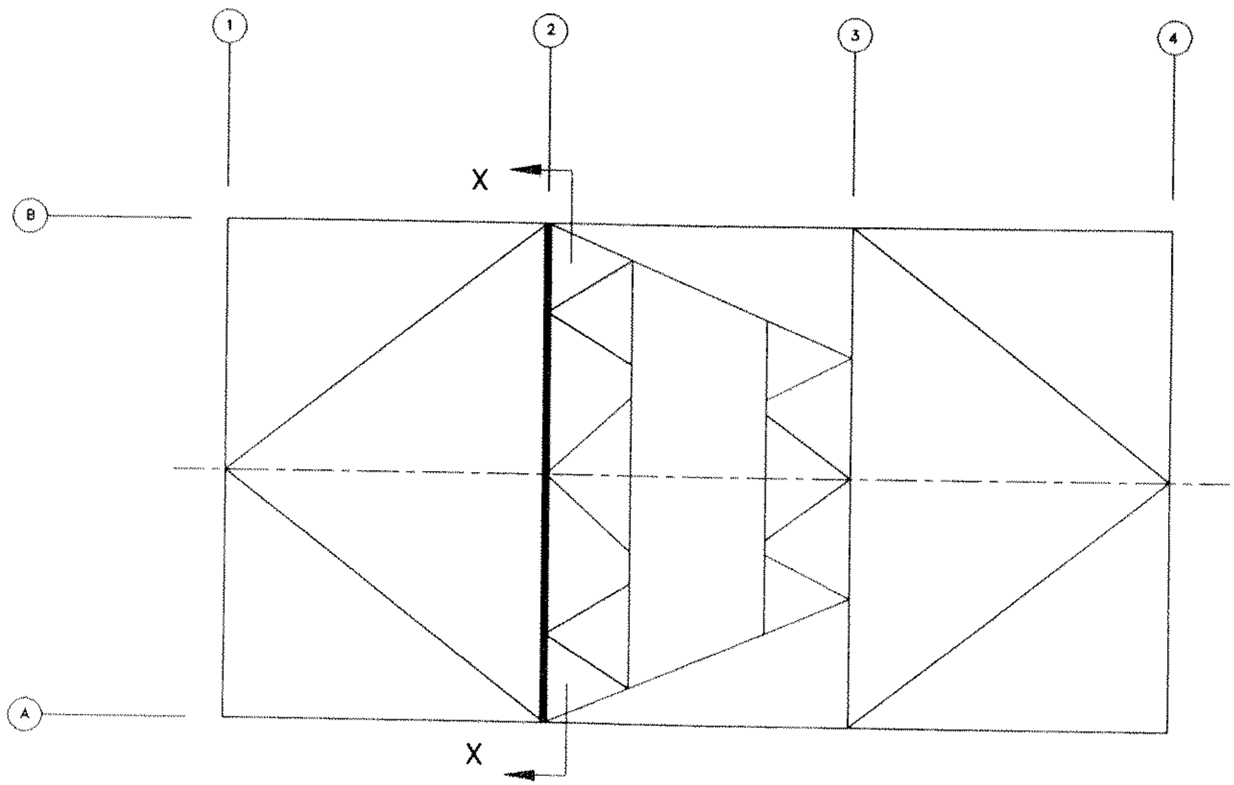


Figure 3.1 PLAN AT ELEVATION −12m LEVEL
SHOWING MEMBER TO BE GROUT–FILLED

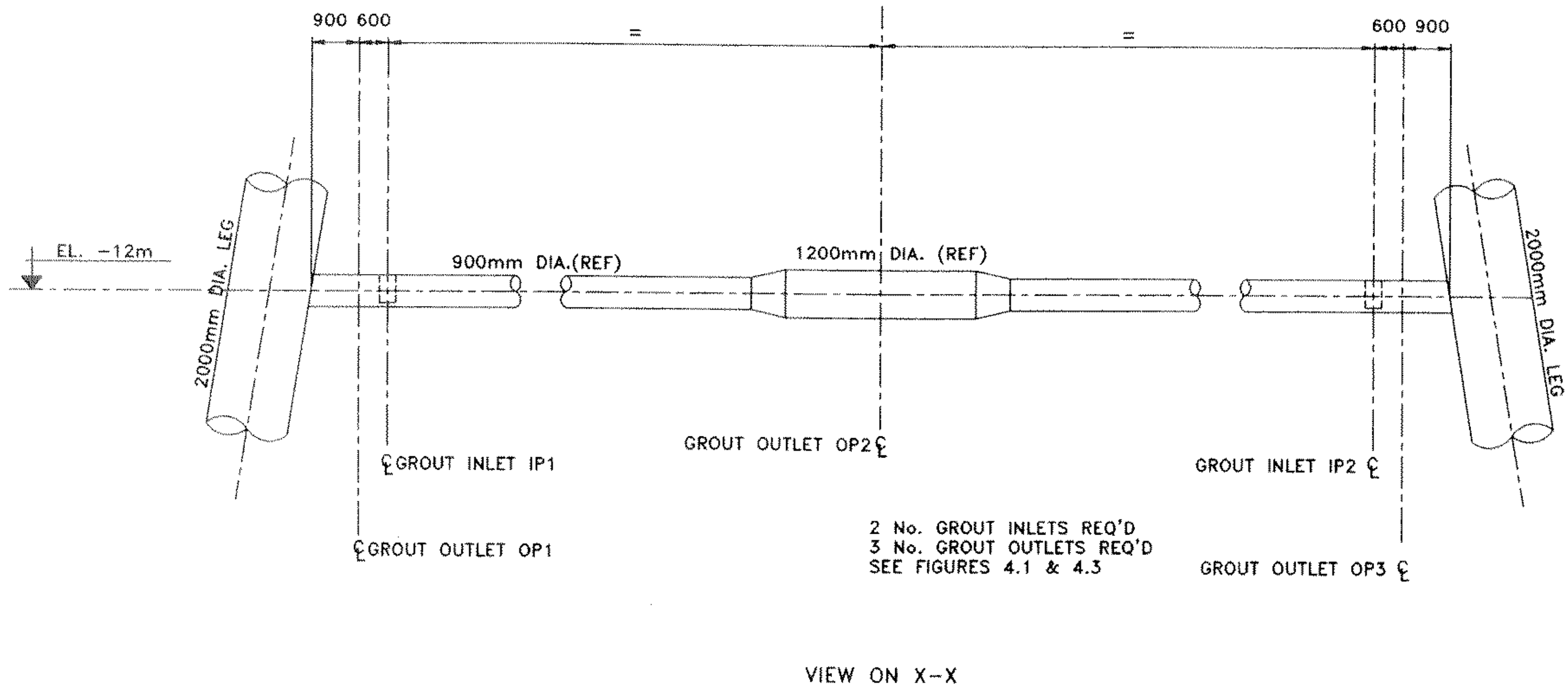


Figure 3.2 MEMBER GROUT FILLING DETAILS

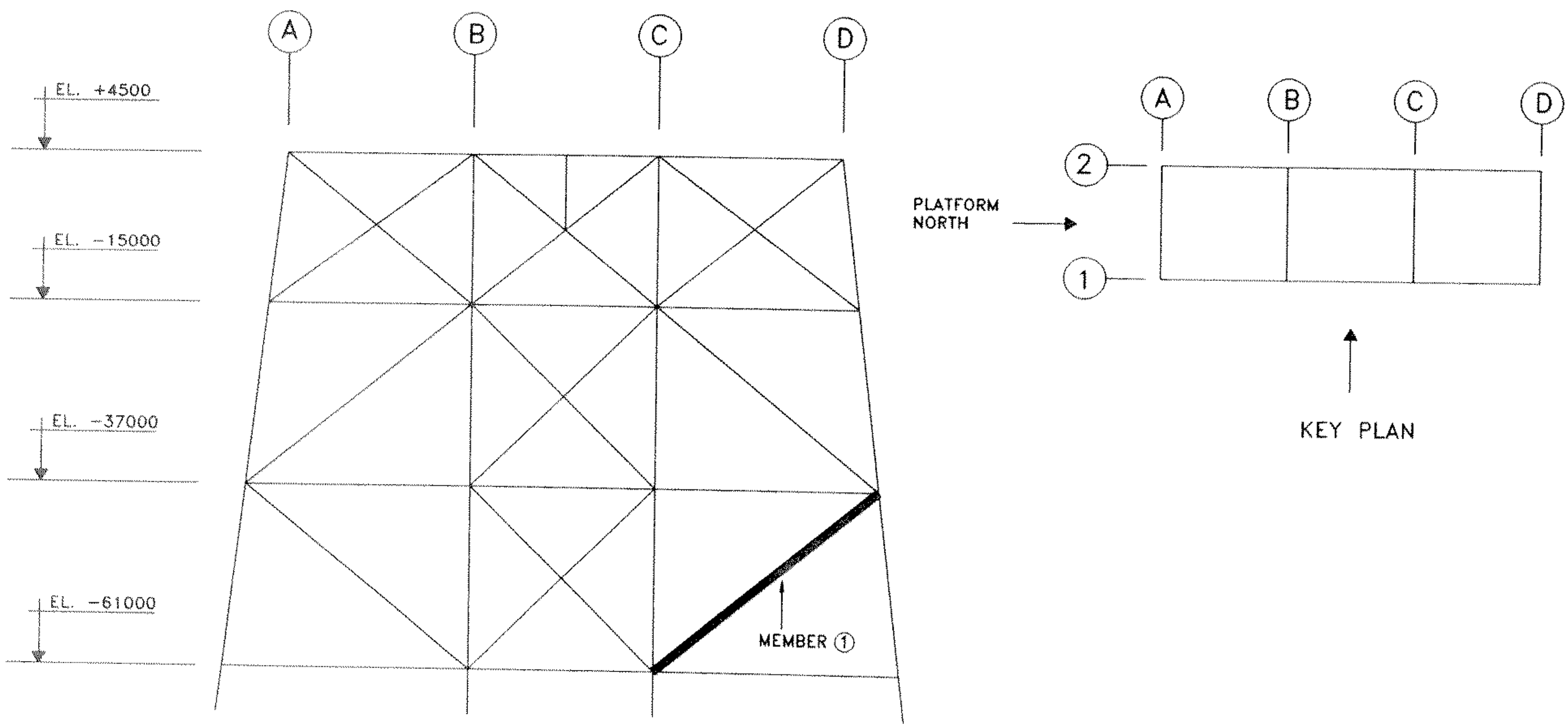


Figure 3.3 ELEVATION ON ROW 1

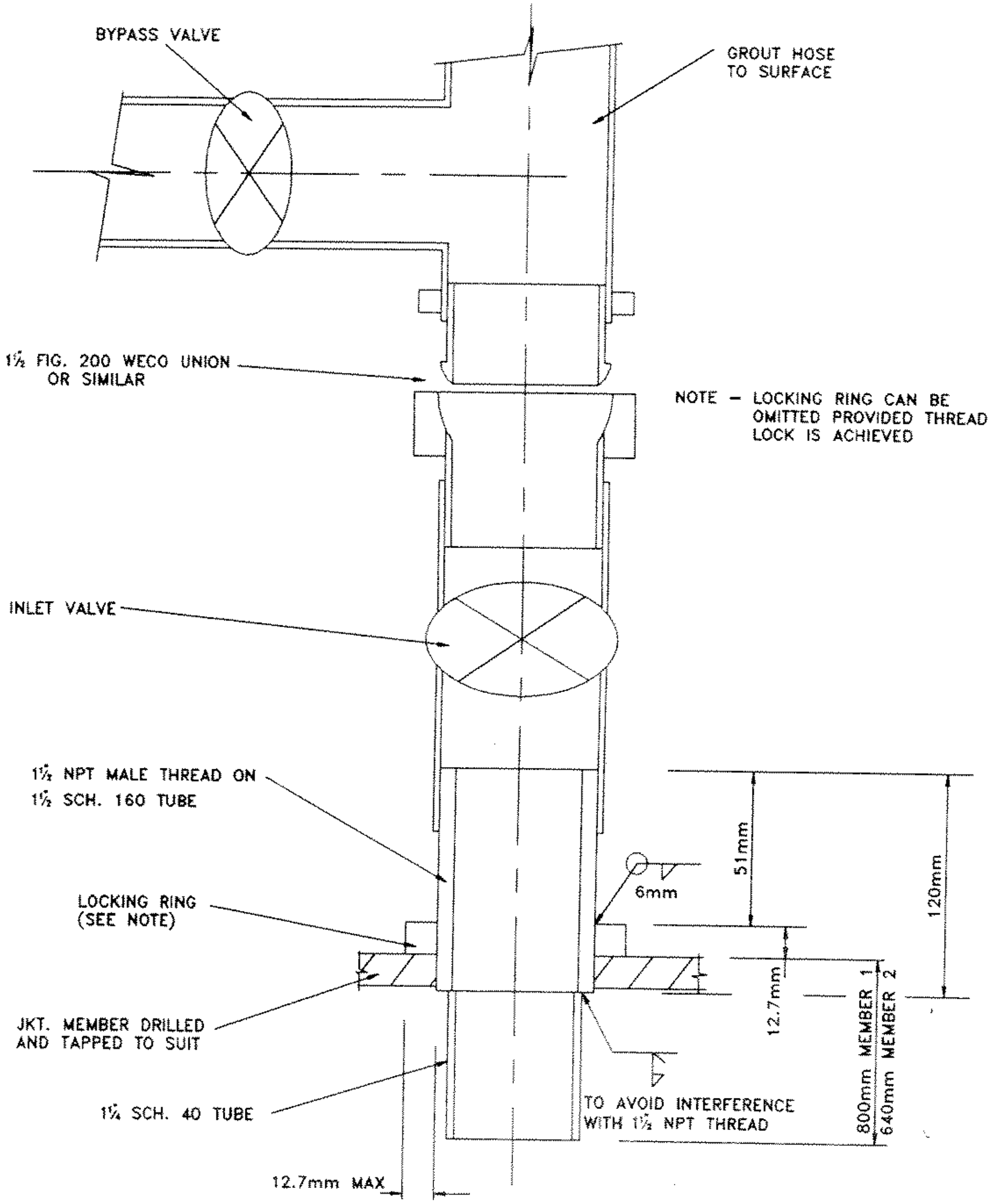
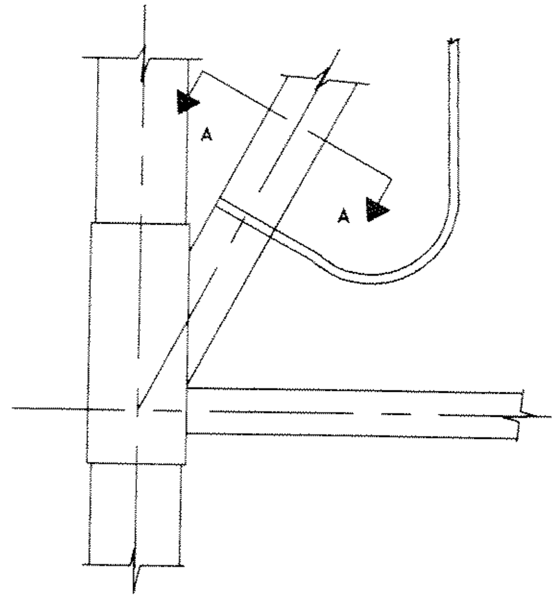


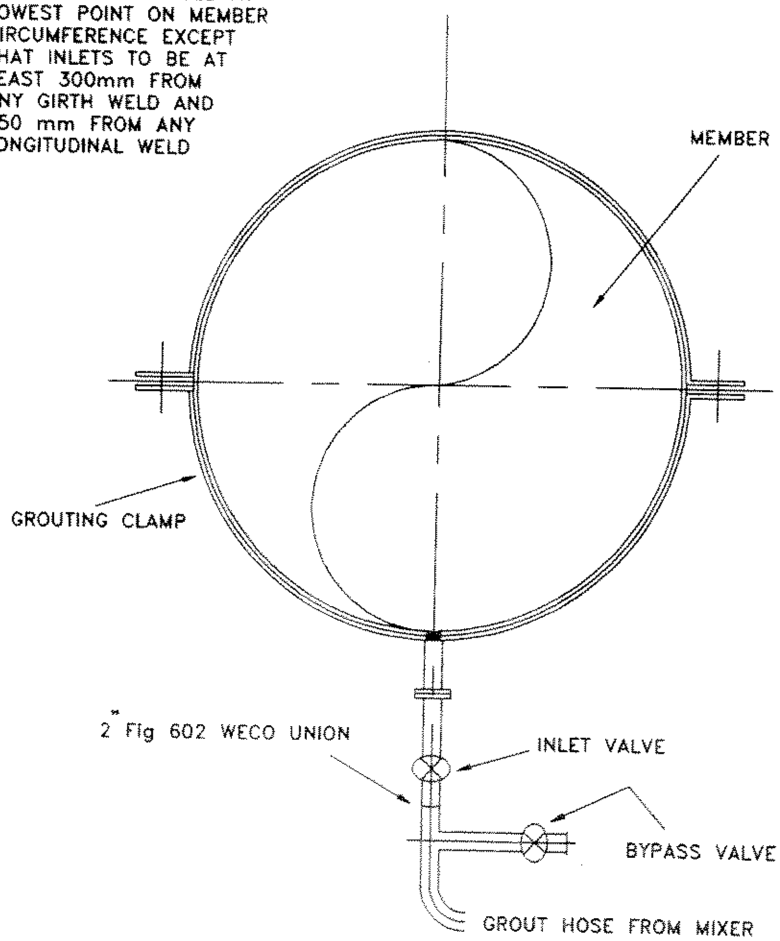
Figure 4.1 ARRANGEMENT OF GROUT INLET CONNECTIONS



ELEVATION

NOTE

INLETS TO BE PLACED AT LOWEST POINT ON MEMBER CIRCUMFERENCE EXCEPT THAT INLETS TO BE AT LEAST 300mm FROM ANY GIRTH WELD AND 150 mm FROM ANY LONGITUDINAL WELD



VIEW A–A

Figure 4.2 DETAIL OF INLET

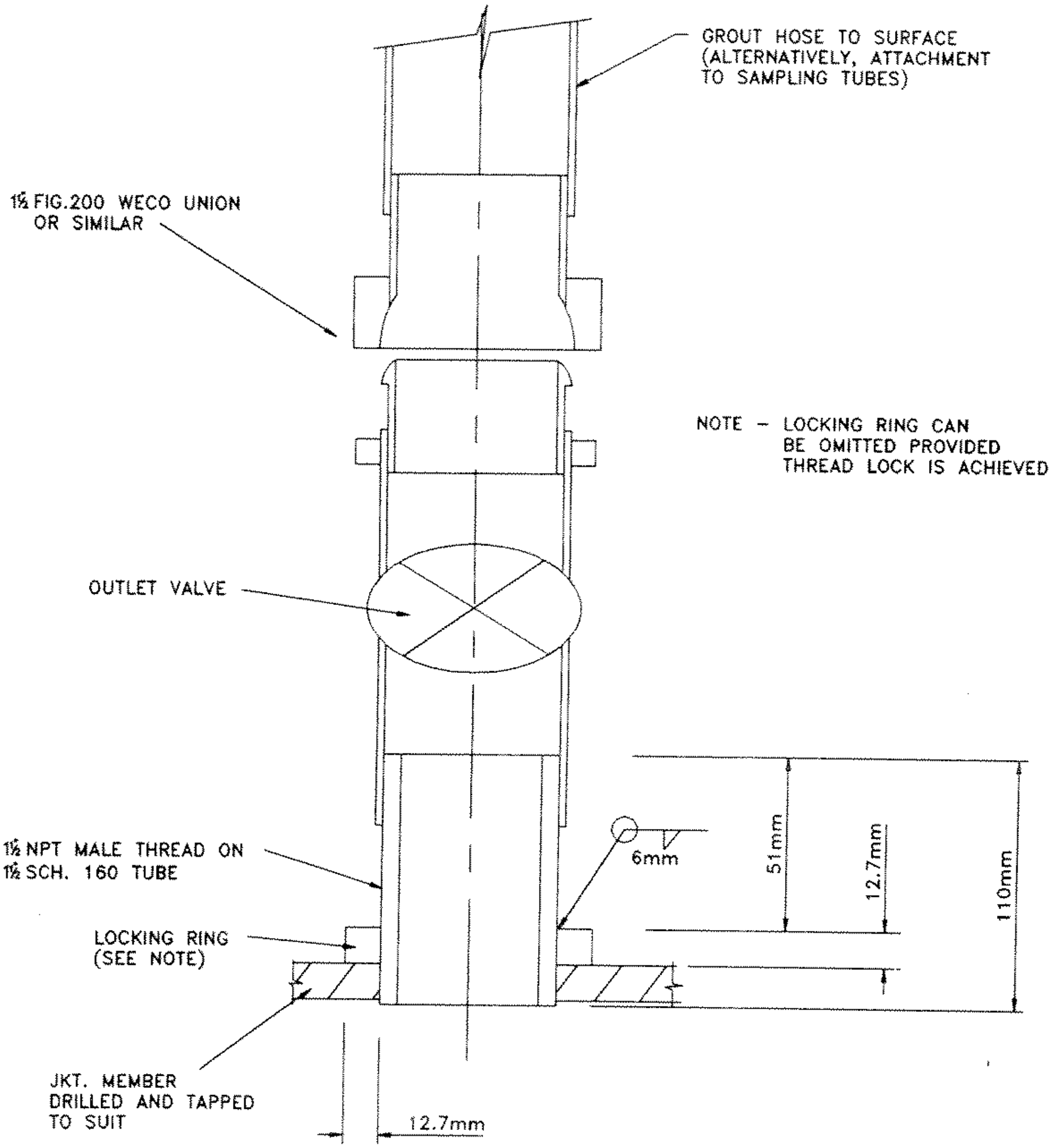


Figure 4.3 ARRANGEMENT OF GROUT OUTLET CONNECTIONS

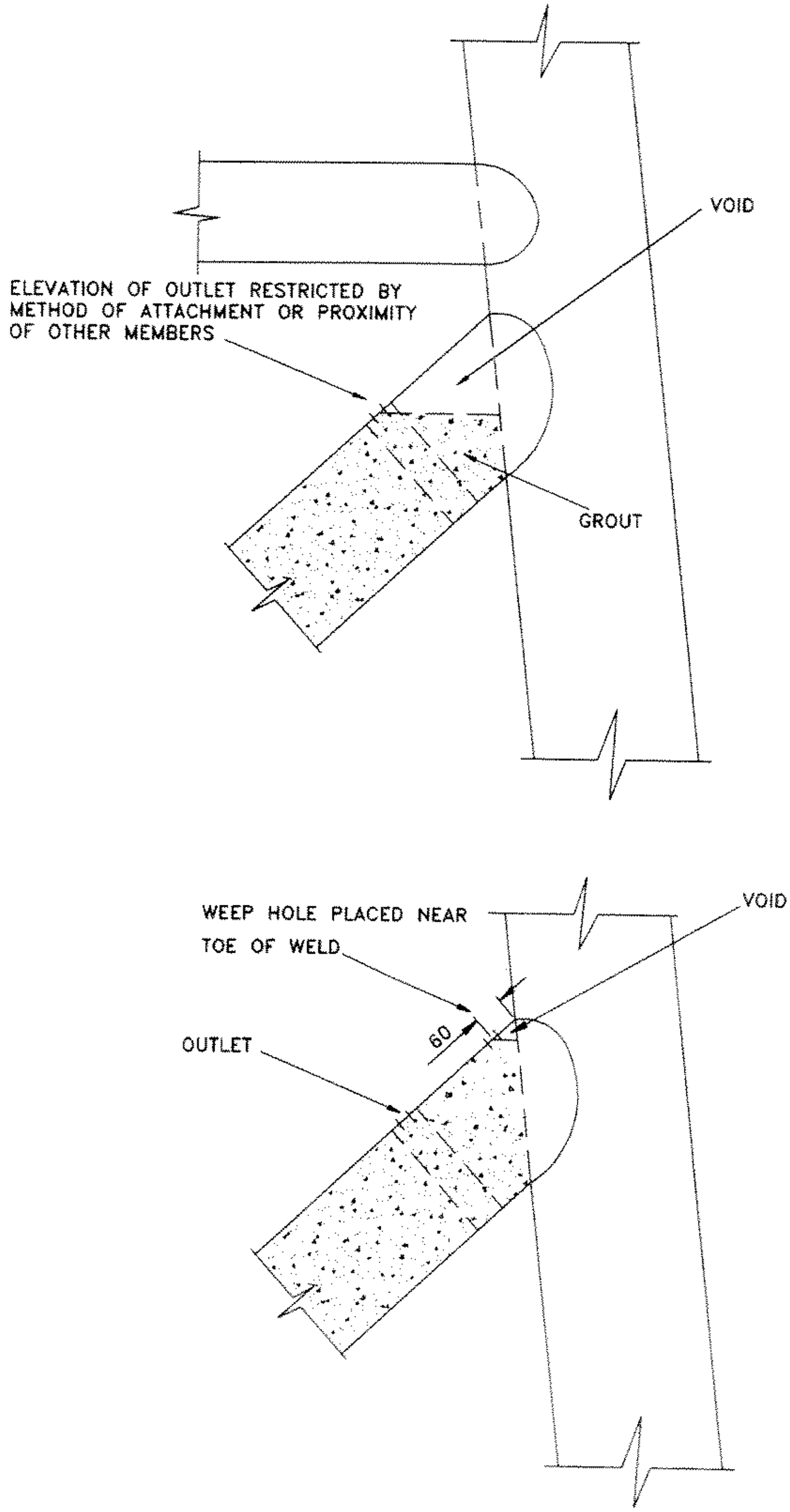
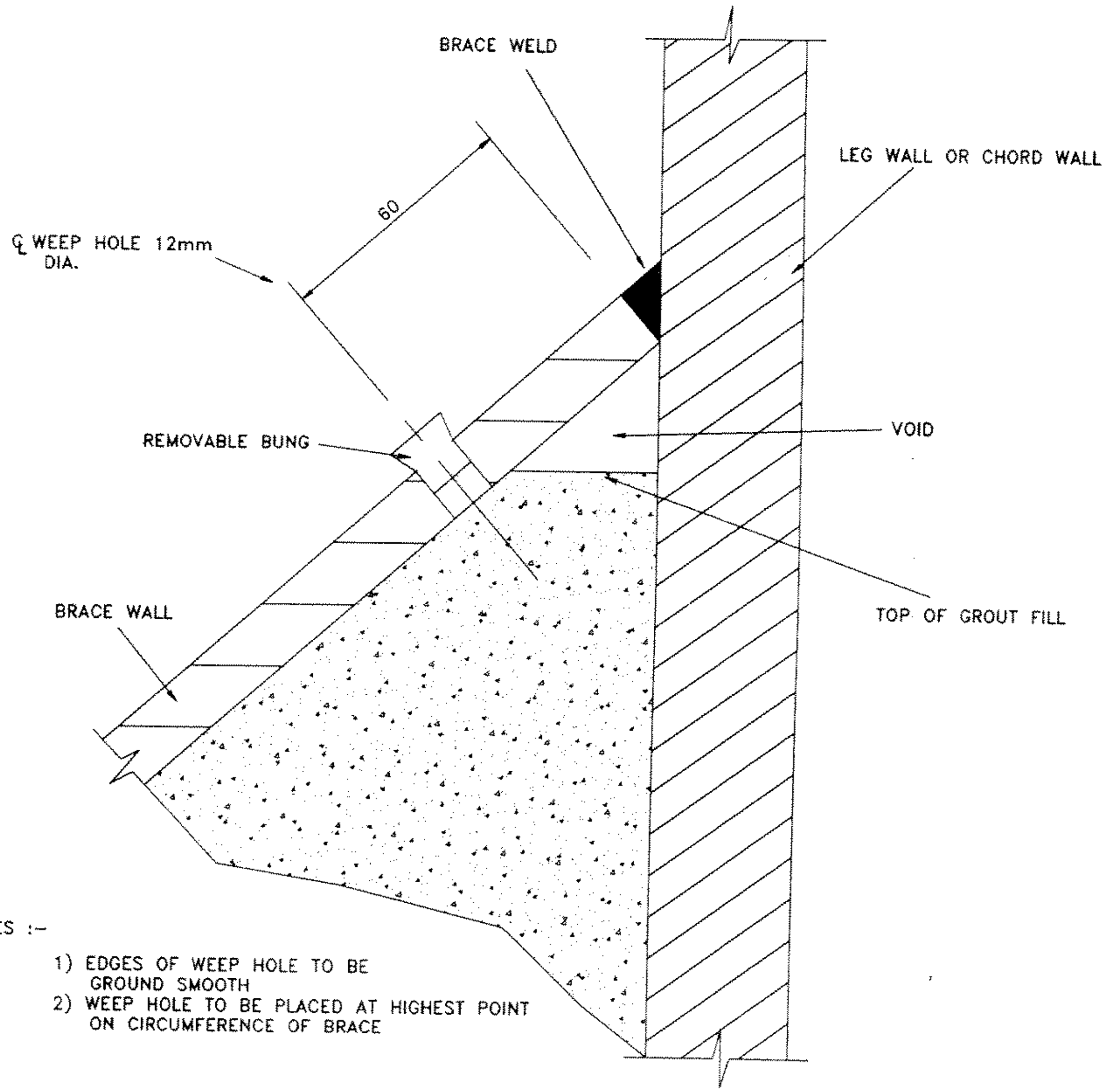


Figure 4.4 – OUTLET DETAILS



NOTES :-

1) EDGES OF WEEP HOLE TO BE GROUND SMOOTH
2) WEEP HOLE TO BE PLACED AT HIGHEST POINT ON CIRCUMFERENCE OF BRACE

Figure 4.5 – WEEP HOLE DETAILS

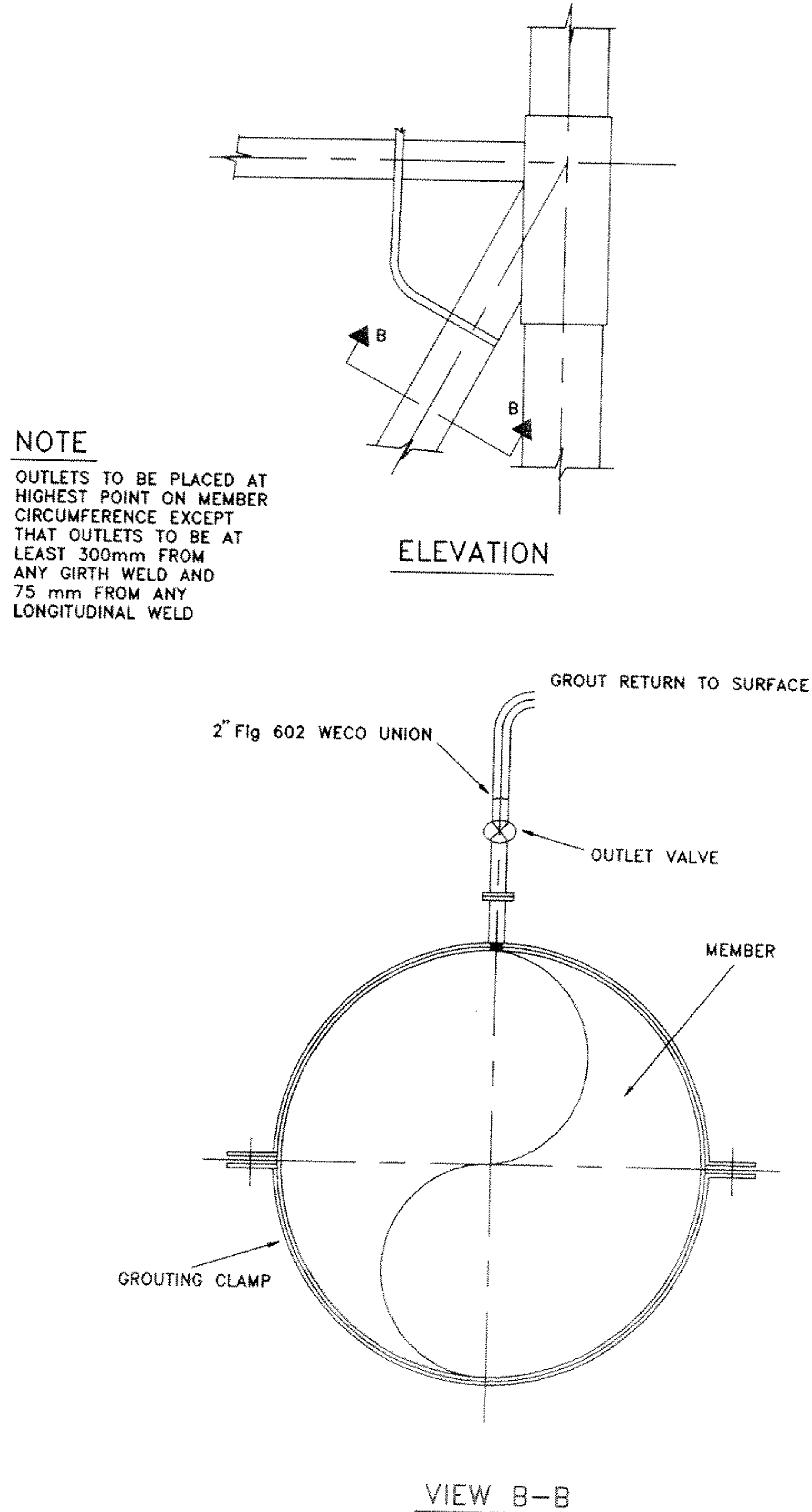


Figure 4.6 DETAIL OF OUTLET

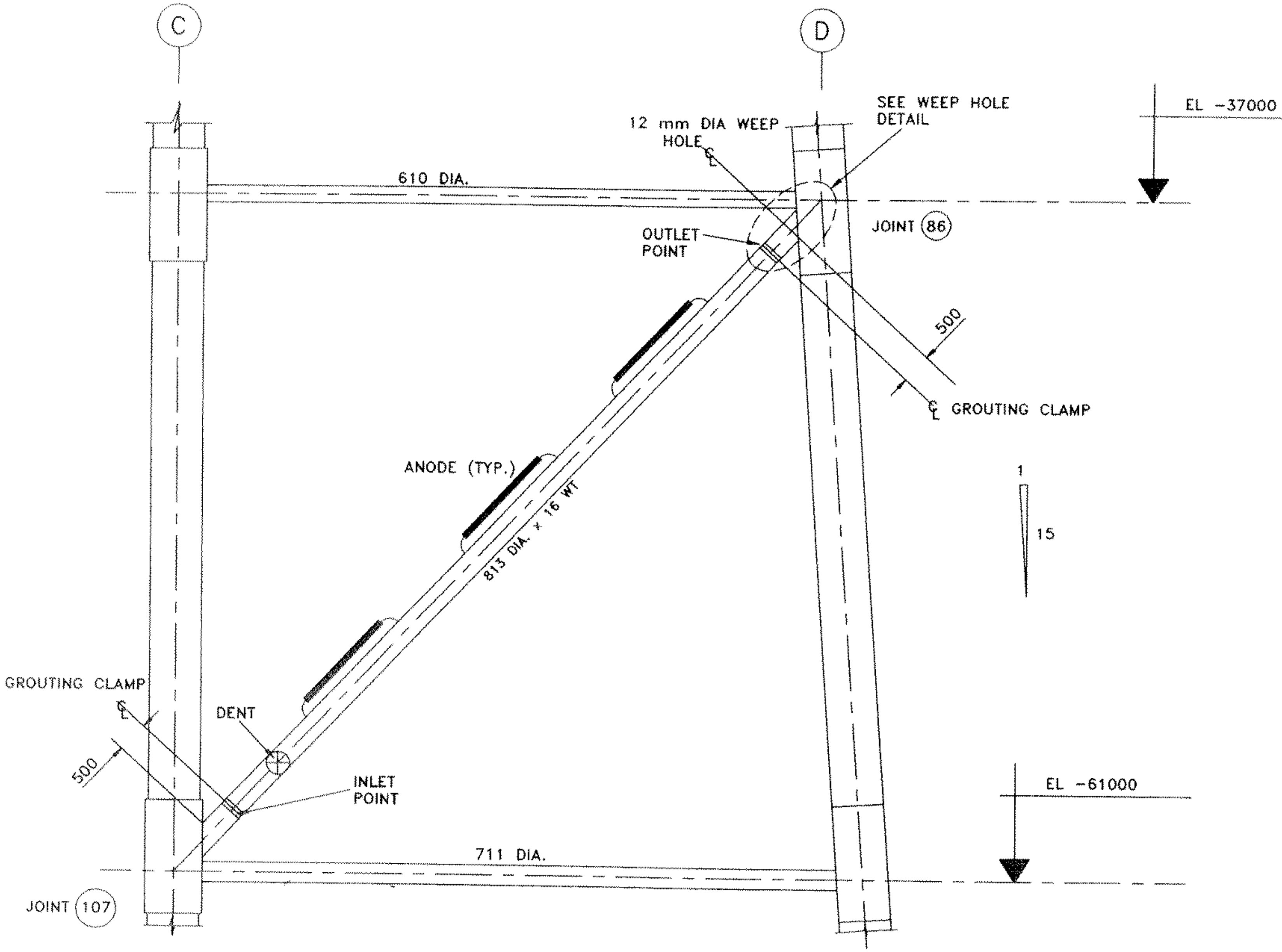


Figure 4.7 ELEVATION ON MEMBER 1

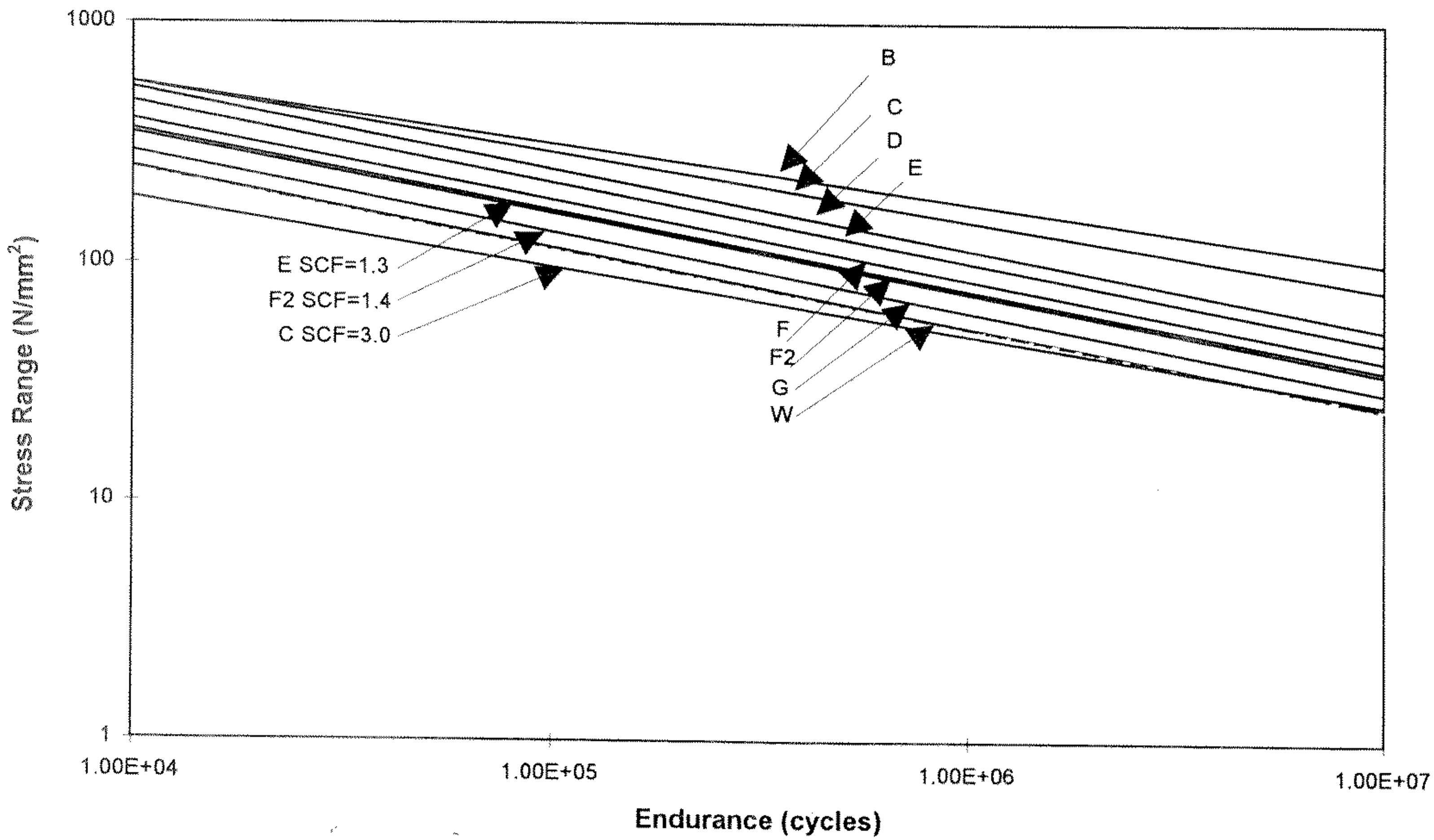


Figure 4.8 FATIGUE LIVES FOR HOLES AND BUTTWELDS IN MEMBERS

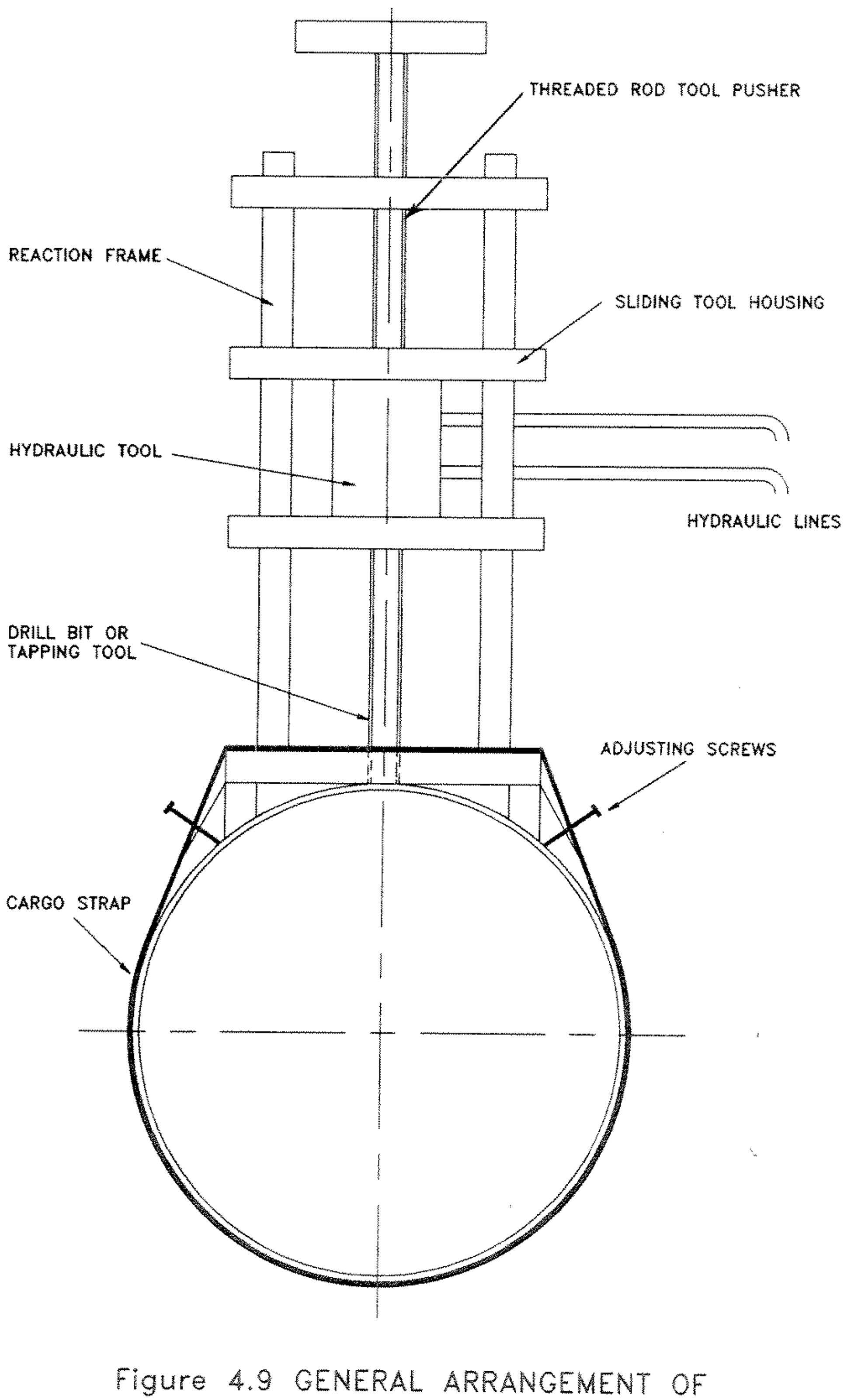


Figure 4.9 GENERAL ARRANGEMENT OF UNDERWATER DRILLING / TAPPING TOOL

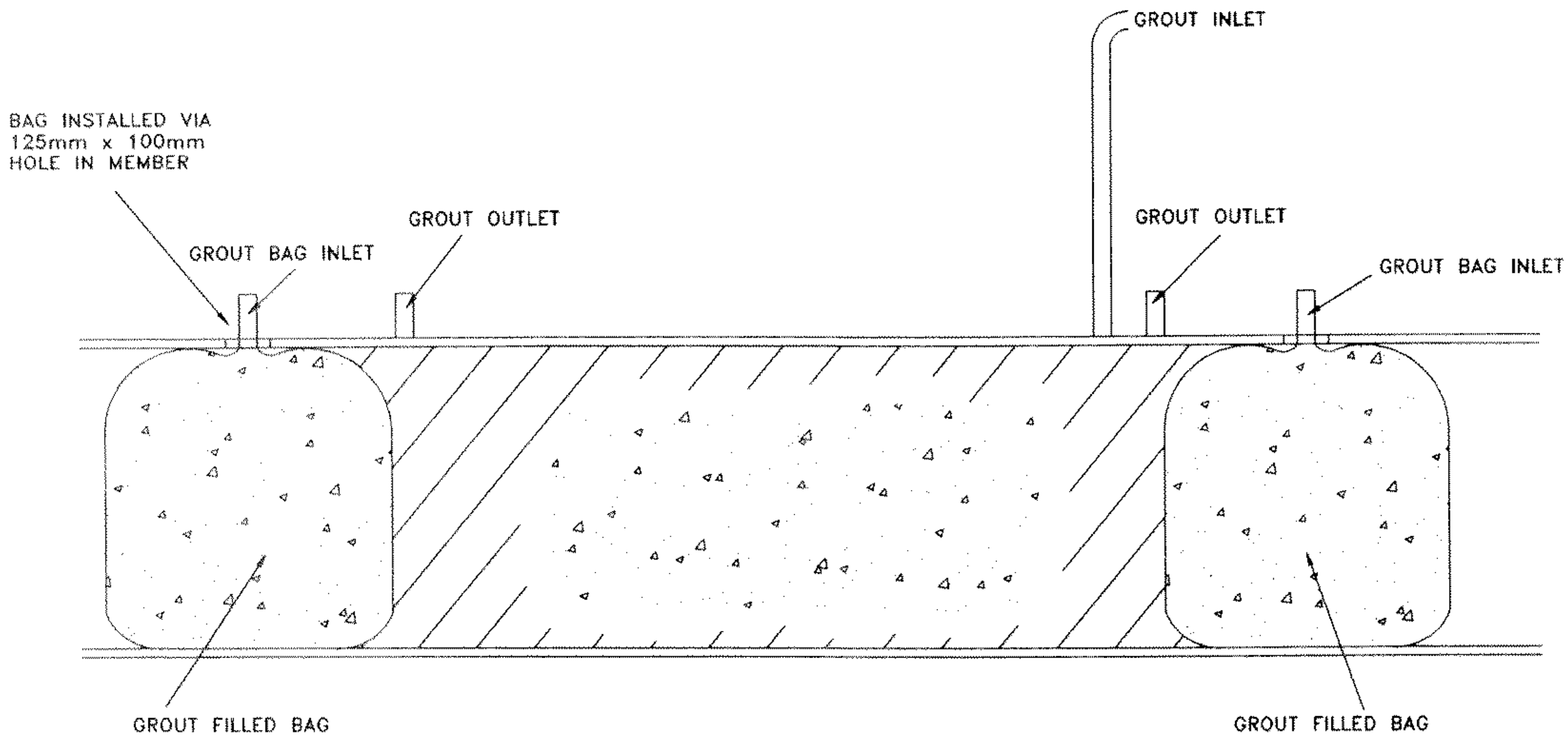


Figure 4.10 APPLICATION OF GROUT BAGS IN MEMBER FILLING